

# MECHANICS OF SOLIDS

Solved Problems

Solved by;

*Masood Akhtar*

NWFP, UET Peshawar

## AISC SPECIFICATION

### FOR STEEL

$$C_c = \sqrt{\frac{2\pi^2 E}{F_y}}$$

If  $C_c < \frac{L_e}{r}$  then  $S_{ux} = \frac{12\pi^2 E}{23 \left(\frac{L_e}{r}\right)^2}$

If  $C_c > \frac{L_e}{r}$  then

$$S_{ux} = \left(1 - \frac{C_c}{\frac{L_e}{r}}\right)^2 \frac{F_y}{F_s}$$

$$F_s = \frac{F_y}{5} + 3 \frac{\left(\frac{L_e}{r}\right)^2}{(2\pi)^2 E}$$

### FOR ALUMINIUM

US Customary

$$S_{ux} = 307 KSE$$

$$\frac{L_e}{r} < 12$$

SI

$$S_{ux} = 1730 MPa$$

$$S_{ux} = (30.7 - 0.25 \frac{L_e}{r}) KSE \quad \frac{L_e}{r} < 12 \quad S_{ux} = (212 - 1.59 \frac{L_e}{r}) MPa$$

$$S_{ux} = \frac{54000 KJ}{\left(\frac{L_e}{r}\right)^2}$$

$$\frac{L_e}{r} > 12$$

$$S_{ux} = \frac{372 \times 10^3 MPa}{\left(\frac{L_e}{r}\right)^2}$$

### FOR WOOD

$S_{ux}$

$$\frac{5.619 E}{\left(\frac{L_e}{r}\right)^2}$$

$$S_{ux} = \frac{0.3 E}{\left(\frac{L_e}{r}\right)^2}$$

$$r = \sqrt{\frac{I}{A}} = \sqrt{\frac{\pi d^4}{4 \times \pi d^2}} = \frac{d}{4}$$

Masood  
Akhtar

CHAPT IICOLOUMNS

$$M = Py \quad (1)$$

$$y = A \cos kx + B \sin kx \quad (2)$$

$$y = e \cos kx + e \tan \frac{kL}{2} \sin kx$$

$$y_{\max} = e \sec \frac{kL}{2}$$

$$S = \frac{P}{A} \left( 1 + \frac{ec}{R} \left( \sec \frac{L}{R} \sqrt{\frac{P}{4AE}} \right) \right)$$

MASOOD AKHTAR  
SKANZ

$$M = Py \Rightarrow y = A \cos kx + B \sin kx$$

$$y = e \cos kx + e \tan \frac{kL}{2} \sin kx$$

$$y_{\max} = e \sec \frac{kL}{2}$$

$$S = \frac{P}{A} \left( 1 + \frac{ec}{R} \left( \sec \frac{L}{R} \sqrt{\frac{P}{4AE}} \right) \right)$$

$$\sin kL = 0$$

$$kL = n\pi$$

$$\sqrt{\frac{P}{EI}} = \frac{n\pi}{L} \Rightarrow \frac{P}{EI} = \frac{n^2 \pi^2}{L^2}$$

$$P = \frac{n^2 \pi^2 EI}{L^2}$$

Critical

$$P_{cr} = \frac{\pi^2 EI}{L^2}$$

Slenderness Ratio  $\frac{L_0}{R} = \sqrt{\frac{AE}{S_{cr}}}$

(2)

GIVEN

PB 134  $D = 1.5\text{m}$ ,  $A = 200\text{mm}^2 = 200 \times 10^{-6}\text{m}^2$

$S_{L\text{max}} = 120 \times 10^6 \text{ Pa}$ ,  $n = 30\text{m}$ ,  $\rho = 1000 \text{ kg/m}^3$

REQUIRED spacing =  $L = ?$

SOL

Since  $P = \rho g h = 1000 \times 9.81 \times 30$

$= 294300 \text{ Pa}$

$F = \rho P = \rho D L$  (small  $\rho$ )

$P = \frac{\rho P L}{2}$

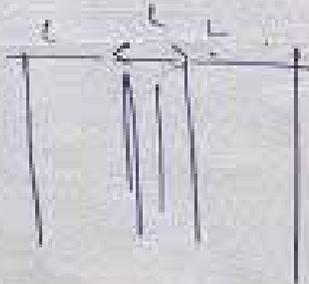
So  $S_{L\text{max}} = \frac{P}{A} = \frac{\rho D L}{2A}$

So  $L = \frac{S_{L\text{max}} \times 2 \times A}{\rho \times D}$

$= \frac{120 \times 10^6 \times 2 \times 200 \times 10^{-6}}{294300 \times 1.5}$

$\underline{L} = \frac{700000}{294300} = 0.176\text{m}$

so spacing =  $0.176\text{m}$



Q. 133 Given (3)

$D = 400 \text{ mm} = 0.4 \text{ m}$   
 $t = 20 \text{ mm} = 0.02 \text{ m}$   
 $P = 4.5 \times 10^6 \text{ Pa}$

REQ  $\Rightarrow$  (a)  $s_t$  &  $s_u$   
 (b)  $P$  if  $s_u$  or  $s_t = 120 \times 10^6 \text{ Pa}$

Sol

(a) Since  
 tangential stress  $= s_t = \frac{PR}{2t}$

$$s_t = \frac{4.5 \times 10^6 \times 0.4}{2 \times 0.02} = 45 \text{ MPa}$$

Since  $s_u = \frac{1}{2} s_t$  ( $s_u = \frac{PR}{4t} = \frac{PR}{2 \times 2t}$ )

$$= \frac{1}{2} \times 45 = 22.5 \text{ MPa}$$

(b) Let  $s_t = 120 \times 10^6 \text{ Pa}$   $\therefore$

$$s_t = \frac{PR}{2t} \Rightarrow P = \frac{s_t \times 2t}{D} = \frac{120 \times 10^6 \times 2 \times 0.02}{0.4}$$

$$P = 12 \text{ MPa}$$

$\therefore$  For  $s_u = 120 \times 10^6 \text{ Pa}$

$$s_u = \frac{PR}{4t}$$

$$P = \frac{s_u \times 4t}{D} = \frac{120 \times 10^6 \times 4 \times 0.02}{0.4}$$

$$P = 24 \text{ MPa}$$

So  $P = 12 \text{ MPa}$  is safe

PD 134

GIVEN  $D = 4 \text{ ft} = 4 \times 12 = 48 \text{ in}$   
 $t = \frac{5}{16} \text{ in}$   
 $S_{\text{max}} = 9000 \text{ psi}$

REQ  $P = ?$

SOL Since for spherical shells

$$S_{\text{max}} = \frac{PD}{4t}$$

So  $P = \frac{S_{\text{max}} \times 4t}{D} = \frac{9000 \times 4 \times \frac{5}{16}}{48}$

$$P = 750.3 \text{ psi}$$


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PD 136

GIVEN  $t = 20 \text{ mm} = 0.02 \text{ m}$ ,  $D = 0.45 \text{ m}$   
 $S_e = 2 \text{ m}$ ,  $S_e = 140 \text{ MPa}$ ,  $S_e = 60 \text{ MPa}$

REQ  $P = ?$

SOL For tangential stress

$$S_e = \frac{PD}{S_e t}$$

$$P = \frac{S_e \times 4t}{D} = \frac{60 \times 10^6 \times 4 \times 0.02}{0.45}$$

$$P = 53 \text{ MPa}$$

For long. stress

$$S_e = \frac{PD}{4t} \Rightarrow P = \frac{S_e \times 4t}{D}$$

$$P = \frac{140 \times 10^6 \times 4 \times 0.02}{0.45} = 240 \rightarrow 24 \text{ MPa}$$

So  $P = 5.3 \text{ MPa} \rightarrow 24 \text{ MPa}$

Pb 137      Given       $D = 22ft = 264in$ ,  $t = \frac{1}{2}in$   
 $S_c = 6000psi$ ,  $\gamma_w = 62.4lb/ft^3$

Req       $h = ?$        $= \frac{62.4}{123} = 0.036$

Sol      since  $P = \rho g h = \gamma_w h$   
 $\& S_c = \frac{PD}{2t} = \frac{\gamma_w h D}{2t}$   
 $h = \frac{S_c \times 2t}{\gamma_w D} = \frac{6000 \times 2 \times 0.5}{62.4 \times 264}$   
 $h = 631.31 in = 52.6ft$

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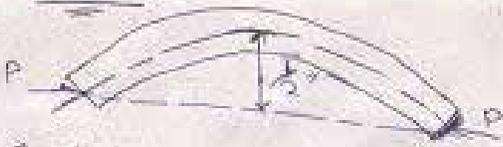
Pb 138

$S_c = 33 kips/ft^2$ ,  $P_s = 150psi$   
 $= \frac{33 \times 10^3}{144}$   
 $0.289 \times 10^3 psi$

$S_c = \frac{PD}{4t}$       \*

41072007)  
PD 902

Case a



Case b



REQD

$$\frac{(S_{max})_{bent}}{(S_{max})_{straight}} = \frac{P}{P}$$

Sol (Case a)

Calculate section properties



$$I = \frac{bh^3}{12} = \frac{1/2 \times 1/2^3}{12} = \frac{0.54 \text{ in}^4}{12}$$

$$C = h/2 = \frac{0.5}{2} = 0.25 \text{ in}$$

$$A = 0.5 \times 0.5 = 0.25 \text{ in}^2$$

$$M = 0.5 \times P$$

Axial stress is  $S_A = P/A = P/0.25 = -4P$

Flexure stress is  $S_f = \frac{Mc}{I} = \frac{0.5P \times 0.25}{\frac{0.54}{12}} = 24P$

Point A is under tension due to flexure

$$\therefore S_A = -S_A + S_f$$

$$= -4P + 24P = 20P$$

AND  $S_B = -4P - 24P = -28P$

Max stress is  $S = -28P$

CASE B straight bar

In this case the bar is subjected to axial stress only and not flexure

$$\therefore S_{max} = -4P$$

$$\text{Now } \frac{(\sigma_{\max})_{\text{bent}}}{(\sigma_{\max})_{\text{straight}}} = \frac{20P}{14P} = 7:1$$

This shows that max stress in bent bar is 7 times than that of straight.

Pb 903  
 20mm  
 40mm  
 150mm  
 500mm

$(\sigma_{ax})_{tension} = 40 \text{ MPa}$   
 $(\sigma_{ax})_{comp} = 80 \text{ MPa}$   
 allowable

Revd  
 $P = ?$   
 safe

Solution  
 Calculate section properties

$I = \frac{bh^3}{12} = \frac{0.04 \times 0.2^3}{12} = 2.67 \times 10^{-6} \text{ m}^4$

$y = 150/100 = 50 \text{ mm} = 0.05 \text{ m}$

$A = bh = 0.04 \times 0.2 = 0.008 \text{ m}^2$

$M = 0.05 \times P = 0.05P$

$c = \frac{200}{2} = 0.1 \text{ m}$

Now Axial stress  $\sigma$

$\sigma_a = -\frac{P}{A} = -\frac{P}{0.008} = -125P$

Flexure stress  $\sigma = \frac{My}{I} = \frac{0.05P \times 0.1}{2.67 \times 10^{-6}} = 187.27P$

PLA is under compression due to flexure stress

$\sigma_A = -\sigma_a + \sigma_f = -125P + 187.27P = 62.27P$

$\sigma_B = -\sigma_a - \sigma_f = -125P - 187.27P = -312.27P$

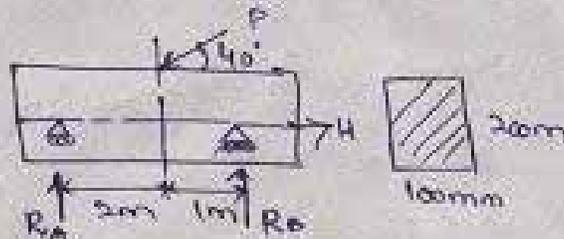
$$\sigma_A = (\sigma_{all})_{comp} = 80 \text{ MPa} = 312.27 \text{ P}$$
$$\therefore P = 256.2 \text{ kN}$$
$$\sigma_B = (\sigma_{all})_{tension} = 4 \text{ MPa} = 62.27 \text{ P}$$
$$P = 642.36 \text{ kN}$$

Safe value of P is least that is 256.2 kN

$\pm P/A = \frac{M}{I}$



PD 905



$\sigma_{max} = 10 \text{ MPa}$  (for both tension and compression)

REQD = First of all resolve P into components

$$P_x = P \cos 40^\circ = 0.766P$$

$$P_y = P \sin 40^\circ = 0.64P$$

Calculate reactions

$$\sum F_y = 0 \text{ +ve } R_A + R_B - P_y = R_A + R_B = 0.64P \quad (1)$$

$$\sum F_x = 0 \text{ +ve } H - P_x = 0 \Rightarrow H = P_x \Rightarrow H = 0.766P$$

$$\sum M_A = 0 \text{ +ve } -R_B \times 3 + R_A \times 2 - P_x \times 0.1 = 0$$

$$R_B = \frac{0.64P \times 2 - 0.766P \times 0.1}{3} = 0.40P$$

$$R_A = 0.64P - 0.40P = 0.24P$$

Calculate section properties

$$I = \frac{bh^3}{12} = \frac{0.1 \times 0.2^3}{12} = 6.67 \times 10^{-5} \text{ m}^4$$

$$A = 0.1 \times 0.2 = 0.02 \text{ m}^2 \quad \left\{ \begin{array}{l} C = h/2 = 0.1 \text{ m} \end{array} \right.$$



$$P_x = 0.766P$$

$$0.24P \times 2 = 0.48P$$

$$R_A = 0.24P$$

Axial stress is Tensile i.e

$$\sigma_a = P/A = \frac{P}{\pi d^2/4} = \frac{4P}{\pi d^2} = \frac{0.766P}{0.02} = 38.3P$$

Flexure stress is  $\sigma_f = \frac{Mc}{I} = \frac{0.40 \times 61}{6.67 \times 10^{-5}} = 719.64P$

Point A is under compression due to moment

$$\sigma_A = +\sigma_a - \sigma_f = 38.3P - 719.64P = -681.34P$$

$\therefore \sigma_A = +\sigma_a - \sigma_f = 38.3P - 719.64P = -681.34P$

But  $\sigma_A = 10 \text{ MPa}$

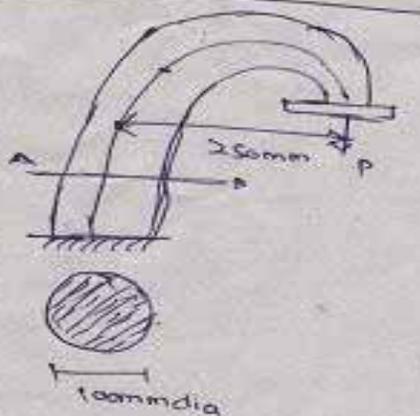
so  $10 \times 10^6 \text{ Pa} = 681.34P = 681.34P$

$$P = 13.1 \text{ kN}$$

Least value is safe value of P is

$$P = 13.1 \text{ kN}$$

Pb 907



$$(\sigma_{lim}) = 10 \text{ MPa}$$

Reqd  $P_{safe} = ?$

Solution calculate section properties

$$I = \frac{\pi d^4}{64} = \frac{\pi \times 0.1^4}{64} = 4.91 \times 10^{-6} \text{ m}^4$$

$$A = \frac{\pi d^2}{4} = \frac{\pi \times 0.1^2}{4} = 7.76 \times 10^{-3} \text{ m}^2$$

$e = d/2 = 0.05\text{m}$   
 Consider equilibrium of section A-B  
 Axial stress is compressive i.e.  

$$S_a = -P/A = -\frac{P}{7.76 \times 10^{-3}} = -129.0P$$

$$S_f = \frac{Mc}{I} = \frac{0.25P \times 0.05}{4.91 \times 10^{-6}}$$

$$S_f = 2545.0P$$

Pt A is under tension due to moment

$$\therefore S_A = -S_a + S_f$$

$$= -129.0P + 2545.0P$$

$$S_A = 2417P \quad \text{put } S_A = 90\text{MPa}$$

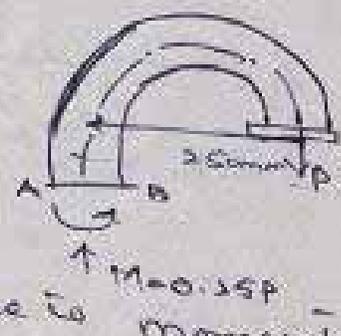
$$90 \times 10^6 \text{ Pa} = 2417P \quad \therefore P = 33 \text{ kN}$$

$$S_B = -129.0P - 2545.0P = -2674.6P$$

$$90 \times 10^6 \text{ Pa} = 2674.6P$$

$$P = 29.9 \text{ kN}$$

safe value of P is 29.9 kN

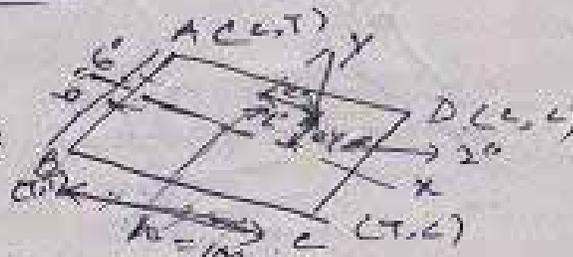


CH#9.  
 (2) KERN OF A SECTION

9/9/2007 (1)

PD 918

Given  $P = 12k$



REQD  
 Stress at each corner

Sol  
 Since general equation for stress is

$$-\frac{P}{A} \pm \frac{M_x}{I_x} y \pm \frac{M_y}{I_y} x.$$

So Dist of all calculating  $M_x, M_y, I_x, I_y$   
 $x, y, A$

$$M_x = P e y = 12 \times 2 = 24 \text{ kip-in} = 24 \times 10^3 \text{ lb-in}$$

$$M_y = P e x = 12 \times 10^3 \times 1 = 12 \times 10^3 \text{ lb-in}$$

$$I_x = \frac{h b^3}{12} = \frac{10 \times 6^3}{12} = 180 \text{ in}^4$$

$$I_y = \frac{b h^3}{12} = \frac{6 \times 10^3}{12} = 500 \text{ in}^4.$$

$$A = 6 \times 10 = 60 \text{ in}^2 \quad x = \frac{h}{2} = \frac{10}{2} = 5 \quad y = \frac{b}{2} = 3$$

So calculating stress at A, B, C, D.

$$\frac{M_x}{I_x} y = 4000 \text{ psi}$$

$$\frac{M_y}{I_y} x = 120 \text{ psi}$$

$$\frac{P}{A} = 12 \times 10^3 / 60 = 200 \text{ psi}$$

$$\sigma_A = -\frac{P}{A} - \frac{M_x}{I_x} y + \frac{M_y}{I_y} x$$

$$= -200 - 400 + 120 = -480 \text{ psi}$$

$$\sigma_B = -P/A + \frac{M_x y}{I_x} + \frac{M_y x}{I_y} = -200 + 400 + 120 = 320 \text{ psi}$$

$$\sigma_C = -P/A + \frac{M_x y}{I_x} - \frac{M_y x}{I_y} = -200 + 400 - 120 = 80 \text{ psi}$$

$$\sigma_D = -P/A - \frac{M_x y}{I_x} - \frac{M_y x}{I_y} = -200 - 400 - 120 = -720 \text{ psi}$$

Diagram illustrating the stress distribution on a rectangular cross-section. The cross-section has a width of 6 inches and a height of 4 inches. The stress distribution is shown as a linear variation across the width, with maximum compression at the top and maximum tension at the bottom. The neutral axis is located 2.4 inches from the top edge.

$$\frac{320}{x} = \frac{480}{6-x}$$

$$1920 - 320x = 480x$$

$$1920 = 800x$$

$$x = 2.4"$$

Neutral axis is at 2.4" along y.

PB 919

since max tensile stress is  $320 \text{ psi}$

so

$$S_B = \frac{P_{add}}{A}$$

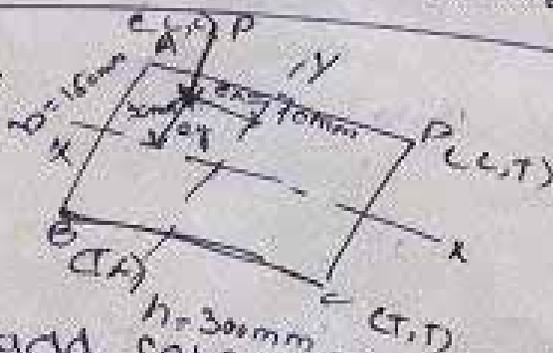
$$\text{or } P_{add} = S_B \times A = 320 \times 60 = 19200 \text{ lbs.}$$

PB 920

$P = 100 \text{ kN}$

RGD

$P_{add} = ?$



To find  $P_{add}$  calculate max  $S_T$  which is at C i.e.

$$S_c = -\frac{P}{A} + \frac{M_x}{I_x} y + \frac{M_y}{I_y} x$$

Calculating  $M_x, I_x, M_y, I_y, A, x, y$ .

$$M_x = 100 \times 10^3 \times 0.03 = 3000 \text{ N-m}$$

$$M_y = 100 \times 10^3 \times 0.075 = 7000 \text{ N-m}$$

$$I_x = \frac{0.03 \times 0.15^3}{12} = 0.00004375 \text{ m}^4$$

$$I_y = \frac{0.15 \times 0.03^3}{12} = 0.000004375 \text{ m}^4$$

$x = 0.15 \text{ m}$   
 $y = 0.075 \text{ m}$   
 $A = 0.045 \text{ m}^2$

$\sigma_{c1} =$   
 so  $P/A = 100 \times 10^3 / 0.045 = 2222222.22$   
 $\frac{M_x}{I_x} y = \frac{3000x}{0.000004375} \times 0.075 = 2666666.667$   
 $\frac{M_y}{I_y} x = \frac{7000x \times 0.075}{0.0003375} = 15311111.111$   
 so  $\sigma_c = -P/A + \frac{M_x}{I_x} y + \frac{M_y}{I_y} x$   
 $= -222222 + 2666666.667 + 15311111.111$   
 $\sigma_c = 3555555.556$   
 so  $\sigma_c = \frac{P_{add}}{A}$   
 so  $P_{add} = 1600000N$

PB 921

$A = 15500 \text{ mm}^2$ ,  $t_w = B \text{ mm}$ ,  $t_f = 21.7 \text{ mm}$   
 $b_f = 257 \text{ mm}$ ,  $S_x = 2010 \times 10^3 \text{ mm}^3$ ,  $S_y = 470 \times 10^3 \text{ mm}^3$

From figure the max  $\sigma$  tensile stress is at B so

$$\sigma_B = -\frac{P}{A} + \frac{M_x}{S_x} + \frac{M_y}{S_y}$$

Putting  $\sigma_B = 0$  to calc kern

$$0 = -\frac{P}{15500} + \frac{P e_y}{S_x} + \frac{P e_x}{S_y}$$

$$\frac{0}{P} = \frac{P}{P} \left( -\frac{1}{15500} + \frac{e_y}{S_x} + \frac{e_x}{S_y} \right)$$

$$0 = -\frac{1}{15500} + \frac{e_y}{S_x} + \frac{e_x}{S_y}$$

or  $\frac{e_y}{2010 \times 10^3} + \frac{e_x}{470 \times 10^3} = \frac{1}{15500}$

$$\frac{e_y}{2010 \times 10^3} \times 15500 + \frac{e_x}{470 \times 10^3} \times 15500 = 1$$

$$\frac{e_x}{470 \times 10^3} + \frac{e_y}{2010 \times 10^3} = \frac{1}{15500}$$

$$\frac{e_x}{30.3} + \frac{e_y}{123.6} = 1$$

Ex 9.23

$$\sigma_x = 0 \text{ MPa} \quad \sigma_y = -40 \text{ MPa}$$

$$\tau_{xy} = \tau_{yx} = \tau \quad \theta = 30^\circ$$

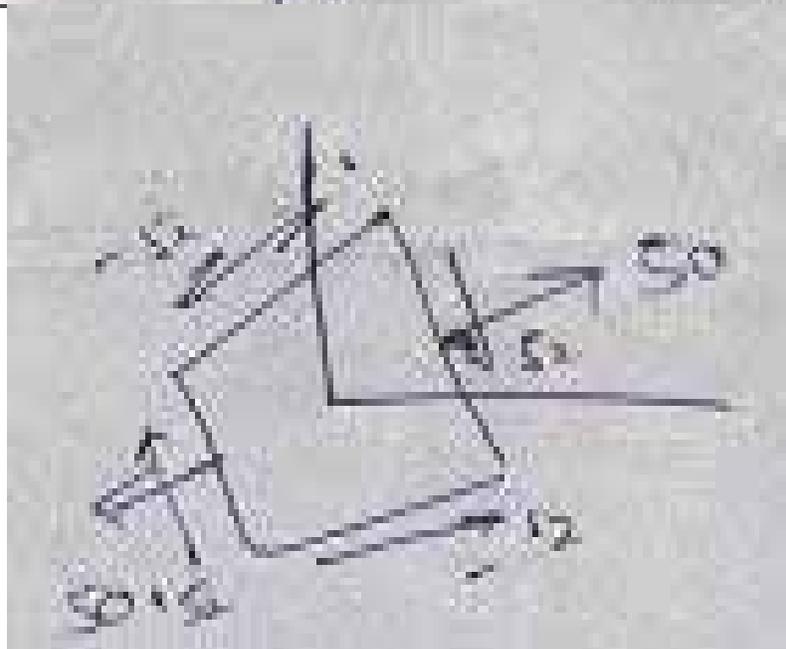
$$\sigma = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta$$

$$= \frac{0 - 40}{2} + \frac{0 + 40}{2} \cos 60 - \tau \sin 60$$

$$= 20 + 20 \cos 60 - \tau \sin 60 = 20 + 10 - \tau \sin 60 = 30 - \tau \sin 60$$

$$\tau = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$$

$$= \frac{0 + 40}{2} \sin 60 + \tau \cos 60 = 20 \sin 60 + \tau \cos 60$$



Pb 924

Determine

Normal &amp; Shearing

(a) principal planes (b) The Plane of max. in plane shearing stress  $\tau$

(c) The planes whose normal are at  $36.8^\circ$  &  $126.8^\circ$  with x-axis

(a) Principal planes.

$$\left. \begin{matrix} \sigma_1 \\ \sigma_2 \end{matrix} \right\} \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$= \frac{32 + (-10)}{2} \pm \sqrt{\left(\frac{32 + 10}{2}\right)^2 + (-20)^2}$$

$$= 11 \pm \sqrt{21^2 + 400} = 11 \pm \sqrt{841}$$

$$\left. \begin{matrix} \sigma_1 \\ \sigma_2 \end{matrix} \right\} = 11 \pm 29 =$$

$$\sigma_1 = 11 + 29 = 40 \text{ MPa} \quad \& \quad \sigma_2 = 11 - 29 = -18 \text{ MPa}$$

$$\tan 2\theta = \frac{-2\tau_{xy}}{\sigma_x - \sigma_y} = \frac{-2(-20)}{32 + 10} = \frac{40}{42}$$

$$2\theta = \tan^{-1} \frac{40}{42} =$$

$$2\theta = 43.6^\circ \quad \rightarrow \quad \theta = 21.8^\circ$$

Now shearing stress

$$\tau = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$$

$$= \frac{32 + 10}{2} \sin 43.6 + (-20) \cos 43.6$$

$$= 14.40 - 14.40 = 0$$

⑥ plane of max in plane shearing stress  
is  $T_{xy}$ ,  $T_{yx}$

$$\begin{aligned} T_{xy \max} &= \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (T_{xy})^2} \\ &= \pm \sqrt{\left(\frac{32 - (-10)}{2}\right)^2 + (-20)^2} \\ &= \pm 39 \end{aligned}$$

$$\tan 2\theta_s = \frac{\sigma_x - \sigma_y}{2T_{xy}} = \frac{32 + 10}{-40} = -\frac{42}{40}$$

$$2\theta_s = -46.3$$

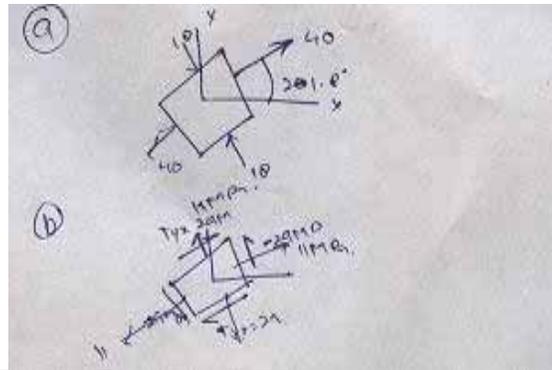
$$\theta_s = -23.15^\circ$$

$$\begin{aligned} \sigma_s &= \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - T_{xy} \sin 2\theta \\ &= \frac{32 - 10}{2} + \frac{32 + 10}{2} \cos 46.3 - (-20) \sin 46.3 \\ &= 11 + 14.5 + 14.5 = 40 \text{ MPa} \end{aligned}$$

⑦

$$\begin{aligned} \sigma_s &= \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - T_{xy} \sin 2\theta \\ &= \frac{32 - 10}{2} + \frac{42}{2} \cos 73.6^\circ - (-20) \sin 73.6^\circ \\ &= 11 + 5.92 + 19.18 = 36.1 \end{aligned}$$

$$\begin{aligned} T &= \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + T_{xy} \cos 2\theta \\ &= \frac{32 + 10}{2} \sin 73.6 + (-20) \cos 73.6 \\ &= 20.1 - 5.64 = 14.45 \text{ MPa} \end{aligned}$$



pb'  
Q25

(3)

$$\sigma_x = \frac{200 \times 10^3}{0.05 \times 0.1} = \frac{200 \times 10^3}{0.005} = 40 \text{ MPa}$$

$$\sigma = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta$$

$$= \frac{40}{2} + \frac{40}{2} \cos(40^\circ) - 0$$

$$= 20 + 20 \cos 40^\circ$$

$$20 + 15.47 = 35.47 = 16.5 \text{ MPa}$$

$$\tau = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta = \frac{40}{2} \sin(40^\circ) = 12.6 \text{ MPa}$$

PB 926RQD (a) Plane stresses (max normal stress)(b) stress components at  $30^\circ$ 

(a)

$$\left. \begin{array}{l} \sigma_1 \\ \sigma_2 \end{array} \right\} \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2}$$

$$= \frac{4000 - 8000}{2} \pm \sqrt{\left(\frac{4000 + 8000}{2}\right)^2 + (6000)^2}$$

$$= -2000 \pm \sqrt{(6000)^2 + (6000)^2} = -2000 \pm \sqrt{72000000}$$

$$= -2000 \pm 8485.28$$

$$\sigma_1 = -2000 + 8485.28 = 6485.28 \text{ psi}$$

$$\sigma_2 = -2000 - 8485.28 = -10485.28 \text{ psi}$$

NOW Stress components

$$\sigma = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta$$

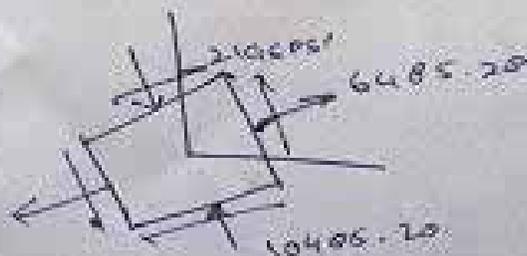
$$= \frac{4000 - 8000}{2} + \frac{4000 + 8000}{2} \cos 60^\circ - (6000 \sin 60^\circ)$$

$$= -2000 + 6000 \times 0.5 + 5196.1 = 6196.1 \text{ psi}$$

$$\tau = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$$

$$= 6000 \times 0.866 + (-6000 \times 0.5)$$

$$5196 - 3000 = 2196 \text{ psi}$$



(b) Now finding components

$$\sin \theta = \frac{c}{r} \Rightarrow \theta = \sin^{-1} \frac{c}{r}$$

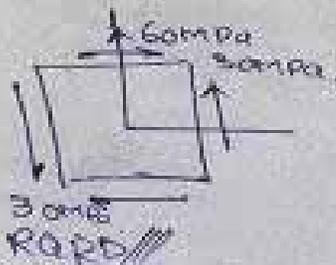
$$\sin \theta = \frac{c}{0.40}$$

$$\theta = 45^\circ$$

$$\sigma_x' = \sigma_x + R \cos 15^\circ = -2 + 0.19 = -1.81 \text{ KSI}$$

$$\tau_{xy}' = R \sin 15^\circ = 0.19 \text{ KSI}$$


Q927



Q Principal stress - ? (b)  $\tau_{max} = ?$

$$\begin{aligned} \sigma_{1,2} &= \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2} \\ &= \frac{60}{2} \pm \sqrt{\left(\frac{-60}{2}\right)^2 + (-30)^2} = 30 \pm \sqrt{(-30)^2 + (-30)^2} \\ &= 30 \pm \sqrt{900 + 900} = 30 \pm 42.426 \end{aligned}$$

$$\sigma_1 = 30 + 42.42 = 72.42 \text{ MPa}$$

$$\sigma_2 = 30 - 42.42 = -12.42 \text{ MPa}$$

Now

$$\begin{aligned} \tau_{max} &= \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2} \\ &= \pm 42.42 \text{ MPa} \quad (\text{as above}) \end{aligned}$$

$$\tan 2\theta_n = \frac{-2\tau_{xy}}{\sigma_x - \sigma_y} = \frac{-2(-30)}{-60}$$

$$\tan 2\theta_n = -2$$

$$2\theta = -63.43$$

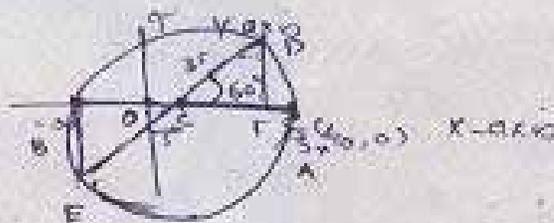
$$\begin{aligned} \tau &= \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta \\ &= -30(-\sin 63.43) + (+30 \cos 63.43) \\ &= 26.03 \text{ MPa} \\ \tau &= 48.24 \end{aligned}$$

PDA20 Given  $\sigma_x = 40 \text{ MPa}$   $\sigma_y = -30 \text{ MPa}$

QD Stress components on planes whose normal are at  $30^\circ$  &  $120^\circ$  with X-axis

By MOHR'S METHOD

- ①  $C = \frac{\sigma_x + \sigma_y}{2} = \frac{40 + (-30)}{2} = 5$
- ②  $R = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2} = \sqrt{\left(\frac{40 - (-30)}{2}\right)^2 + 0} = \sqrt{35^2} = \sqrt{1225} = 35 \text{ MPa}$
- ③ coordinates are  $(40, 0)$



So stress components at  $60^\circ$  are

$$\sigma' = OC + CD = 5 + 35 = 40 \text{ MPa}$$

$$= OC + EF = 5 + 35 \cos 60 = 22.5$$

$$\sigma'' = OC - 35 \cos 60 = 5 - 17.5 = -12.5 \text{ MPa}$$

$$\tau_{60} = 35 \sin 60 = 30.3 \text{ MPa}$$

$$\tau_{120} = -30.3 \text{ MPa}$$



pb  
Q20

$$\sigma_x = 40 \text{ MPa} \quad \& \quad \sigma_y = -30 \text{ MPa}$$

compute stress components

$$\sigma = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau \sin 2\theta$$

$$= \frac{40 + (-30)}{2} + \frac{40 - (-30)}{2} \cos 60^\circ - 0$$

$$\left\{ \begin{aligned} \tau &= \frac{\sigma_x - \sigma_y}{2} \sin 2\theta = \frac{40 - (-30)}{2} \sin 60^\circ = 30 \cdot 0.866 = 25.98 \text{ MPa} \\ &= 26 \text{ MPa} \end{aligned} \right.$$

and  $\theta = 480^\circ$

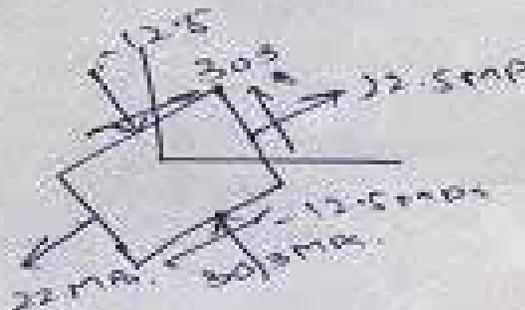
$$\sigma = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta$$

$$= \frac{40 - 30}{2} + \frac{40 + 30}{2} \cos 240^\circ$$

$$\sigma = 5 - 17.5 = -12.5 \text{ MPa}$$

$$\& \quad \tau = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta = \frac{40 + 30}{2} \sin 240^\circ$$

$$= -30 \cdot 0.866 = -25.98 \text{ MPa}$$



Q.29 RQD stress components at  $30^\circ$  &  $120^\circ$

$$\sigma_{1,2} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2} = 0 \pm \sqrt{0 + (-0.000)^2}$$

$$\sigma = 0.000 = 0 \text{ kN/m}^2$$

$$\sigma_1 = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta$$

$$0 + 0 - (-0.000) \sin 60$$

$$\sigma_1 = + 692.0 \text{ MPa}$$

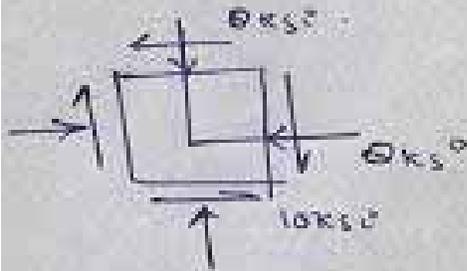
and a)  $\sigma_{120} = -\tau_{xy} \sin 240^\circ$

$$= -(-0.000) \sin 240^\circ$$

$$= - 692.0 \text{ MPa}$$

Pb Q31

QAD normal & shearing stress on the planes whose normal are  $60^\circ$  &  $150^\circ$



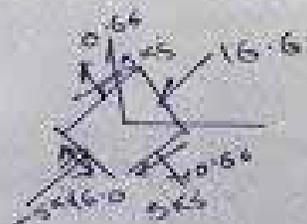
$$\begin{aligned}\sigma_{60} &= \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta \\ &= \frac{-8 - 8}{2} + \frac{-8 + 8}{2} \cos 120^\circ - (10) \sin 120^\circ \\ &= -8 + 0 - 8.66 = -16.66 \text{ KSE}\end{aligned}$$

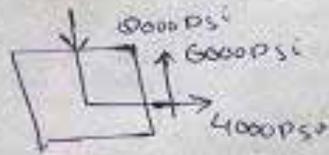
$$\begin{aligned}\sigma_{150} &= \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 300^\circ - \tau_{xy} \sin 300^\circ \\ &= \frac{-8 - 8}{2} + \frac{-8 + 8}{2} \cos 300^\circ - (10) \sin 300^\circ \\ &= -8 + 0 + 8.66 = 0.66 \text{ KSE}\end{aligned}$$

and

$$\begin{aligned}\tau_{60} &= \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta \\ &= \frac{-8 + 8}{2} \sin 120^\circ + (10) \cos 120^\circ \\ &= -5 \text{ KSE}\end{aligned}$$

$$\tau_{150} = 0 + (10) \cos 300^\circ = 5 \text{ KSE}$$



P0932Q90 Max-in plane shearing stressMOHR METHOD

$$\textcircled{1} \quad C = \frac{\sigma_x + \sigma_y}{2} = \frac{4000 + 0000}{2} = -2000 \text{ psi}$$

$$= -2 \text{ ksi}$$

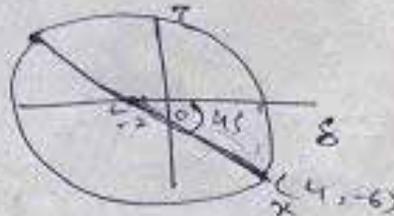
$$\textcircled{2} \quad R = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2} = \sqrt{\left(\frac{4000 + 0000}{2}\right)^2 + (-6000)^2}$$

$$= \sqrt{(6000)^2 + (-6000)^2} = \sqrt{36000000 + 36000000}$$

$$= 0.40520 \cdot$$

$$0.40 \text{ ksi}$$

\textcircled{3} co-ordinates are  $(4, -6)$



Since Max in plane shearing stress is equal to R

So  $\tau_{\max} = 0.40 \text{ ksi}$

Ans

$$\tan 2\theta = \frac{-2\tau_{xy}}{\sigma_x - \sigma_y} = \frac{-2(-6)}{4 + 0} = \frac{12}{4} = 3$$

$$\theta = 22.5$$

Pb 933 (9)

(a) PR-stress    (b) Max. in plane shear  
 (c) stress components at  $45^\circ$  &  $135^\circ$

(a)

$$\left. \begin{array}{l} \sigma_1 \\ \sigma_2 \end{array} \right\} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2}$$

$$= \frac{-60 + 60}{2} \pm \sqrt{\left(\frac{-60 - 60}{2}\right)^2 + (40)^2}$$

$$= 0 \pm \sqrt{(-60)^2 + (40)^2} = \pm \sqrt{3600 + 1600} = \pm 72.11 \text{ MPa}$$

$$\sigma_1 = 72.11 \text{ MPa} \quad \& \quad \sigma_2 = -72.11 \text{ MPa}$$

(b)  $\tau_{\max} = \pm 72.11 \text{ MPa}$   
 $\tau_{\max} = 72.11 \text{ MPa}$

(c)

$$\sigma_{45^\circ} = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$= \frac{-60 + 60}{2} + \frac{-60 - 60}{2} \cos 90^\circ + 40 \sin 90^\circ$$

$$= 0 + 0 + 40 = 40 \text{ MPa}$$

$$\sigma_{135^\circ} = 0 + \frac{-60 - 60}{2} \cos 270^\circ + 40 \sin 270^\circ$$

$$= -40 \text{ MPa}$$

$\tau_{45^\circ} = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$

$$= \frac{-60 - 60}{2} \sin 90^\circ + 40 \cos 90^\circ$$

$$= -60 \text{ MPa}$$

$\tau_{135^\circ} = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$

$$= \frac{-60 - 60}{2} \sin 270^\circ + 40 \cos 270^\circ$$

$$= +60 \text{ MPa}$$

Pb 934

$\sigma_x = \frac{P_x}{A_x} = \frac{24000}{1.6 \times 1.2} = 12500 \text{ PSI}$   
 $\sigma_y = \frac{12000}{1.6 \times 1.2} = 4000 \text{ PSI}$

and

$\tan \theta = \frac{1.6}{1.2} \Rightarrow \theta = \tan^{-1} 1.33$   
 $\theta = 53.1^\circ$

$$\epsilon_x = \frac{2400}{1.2 \times 0.2} = 10 \text{ K}\epsilon$$

$$\epsilon_y = \frac{1200}{1.6 \times 0.2} = 4 \text{ K}\epsilon$$

$$\sigma_{AB} = \frac{10+4}{2} + \frac{10-4}{2} \cos 106.2$$

$$= 7 + 3(7 - 0.03) = 6.16 \text{ K}\epsilon$$

$$\tau_{AB} = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta = \frac{10-4}{2} \sin 106.2 = 2.80 \text{ K}\epsilon$$

• PB Q35

$$S_1 = \frac{S_x + S_y}{2} + \sqrt{\left(\frac{S_x - S_y}{2}\right)^2 + (T_{xy})^2}$$

$$S_2 = \frac{S_x + S_y}{2} - \sqrt{\left(\frac{S_x - S_y}{2}\right)^2 + (T_{xy})^2}$$

Adding ① & ②

$$S_1 + S_2 = S_x + S_y$$

$$2000 - 0000 = S_x + S_y$$

$$-6000 = S_x + S_y \rightarrow \text{①}$$

$$-6 = S_x + S_y$$

Now subtracting.

$$S_1 - S_2 = 0 \sqrt{\left(\frac{S_x - S_y}{2}\right)^2 + (S_y)^2}$$

$$\frac{2000}{2 + 0} = 2 \sqrt{\frac{(S_x - S_y)^2}{4} + 3^2}$$

$$10 = 2 \sqrt{\frac{(S_x - S_y)^2 + 36}{4}}$$

$$10 = \sqrt{(S_x - S_y)^2 + 36}$$

$$100 = S_x - S_y + 36$$

$$100 - 36 = S_x - S_y$$

$$64 = S_x - S_y \rightarrow \text{②}$$

Adding ① & ②

$$64 = S_x - S_y$$

$$-6 = S_x + S_y$$


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$$50 = 2S_x$$

$$S_x = 25 \text{ Ks/c}$$

So  $64 = 25 - S_y$

$$S_y = 25 - 64$$

$$S_y = -39 \text{ Ks/c}$$

PB Q41  $\sigma_1 = 50 \text{ ksi}$ ,  $\sigma_2 = 20 \text{ ksi}$

P. Max. in Plane Shearing Stress

$$\tau = \frac{|\sigma_1 - \sigma_2|}{2} = \frac{50 - 20}{2} = 15 \text{ ksi}$$

∴ Abs Max

$$\tau = \frac{|\sigma_1 - \sigma_2|}{2} = 15 \text{ ksi}$$

$$\tau_{\max} = \frac{|\sigma_1|}{2} = 25 \text{ ksi} \quad \text{or} \quad \tau_{\max} = \frac{|\sigma_2|}{2} = 10 \text{ ksi}$$

∴  $\tau_{\max} = 25 \text{ ksi}$

PB Q43  $\sigma_1 = 8 \text{ ksi}$ ,  $\sigma_2 = 2 \text{ ksi}$

Max in Plane Shearing Stress =  $\frac{|\sigma_1 - \sigma_2|}{2} = 1.5 \text{ ksi}$

(b)  $\tau_{\max} = \frac{|\sigma_1 - \sigma_2|}{2} = \frac{6}{2} = 1.5 \text{ ksi}$

$$\tau_{\max} = \frac{|\sigma_1|}{2} = 2.5 \text{ ksi} \quad \text{or} \quad \tau_{\max} = \frac{|\sigma_2|}{2} = 1 \text{ ksi}$$

Pr 944

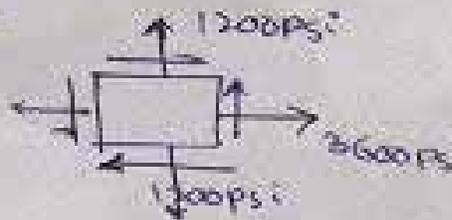
(a) Max in plane  $\tau = \frac{\sigma_1 - \sigma_2}{2} = \frac{-40 + 0}{2} = 20 \text{ MPa}$

(b)  $\tau_{\text{max}} = \frac{-40 + 0}{2} = 20 \text{ MPa}$

$\tau_{\text{max}} = \frac{-40}{2} = 20 \text{ MPa}$

$\tau_{\text{max}} = \frac{-0}{2} = 0 \text{ MPa}$

Pb 946



find of all finding  $\sigma_1$  &  $\sigma_2$

$$\begin{aligned}\frac{\sigma_1}{\sigma_2} &= \frac{3600 + 1200}{2} \pm \sqrt{\left(\frac{3600 - 1200}{2}\right)^2 + (T_{xy})^2} \\ &= 2400 \pm \sqrt{1440000 + 140000} \\ &= 2400 \pm 1697\end{aligned}$$

$$\sigma_1 = 2400 + 1697 = 4097 \text{ psi}$$

$$\sigma_2 = 703 \text{ psi}$$

so (a) Maxin plane Shearing stress

$$\tau = \frac{|\sigma_1 - \sigma_2|}{2} = \frac{4097 - 703}{2} = 1697 \text{ psi}$$

(b)

$$\tau_{max} = \frac{|\sigma_1 - \sigma_2|}{2} = 1697 \text{ psi}$$

$$\tau_{\sigma_1} = \frac{|\sigma_1|}{2} = \frac{4097}{2} = 2048.5 \text{ psi}$$

$$\tau_{\sigma_2} = \frac{|\sigma_2|}{2} = \frac{703}{2} = 351.5 \text{ psi}$$

Pb 947-949 Same as above

Given  $E = 10 \times 10^7 \text{ Pa}$ ,  $S_{yp} = 30 \times 10^6 \text{ Pa}$   
 F.O.S. = 2,  $L = 2.5 \text{ m}$

REQ (a) Minimum length  
 (b)  $P_{all} = ?$



Sol

$$\frac{L_e}{R} = \sqrt{\frac{\bar{\lambda}^2 E}{S_{yp}}} = \sqrt{\frac{3.14^2 \times 10 \times 10^7}{30 \times 10^6}}$$

$$\frac{L_e}{R} = \sqrt{32.065} = 5.732$$

$$I = \frac{100 \times 50^3}{12} = 1041666.667 \text{ mm}^4$$

$$A = 5000 \text{ mm}^2$$

$$I = AR^2 \Rightarrow R = \sqrt{\frac{I}{A}} = \sqrt{\frac{1041666.667}{5000}}$$

$$R = 14.43 \text{ mm}$$

$$\frac{L_e}{R} = 5.732 \Rightarrow L_e = 5.732 \times 14.43$$

$$L_e = 82.734 \text{ mm}$$

$$L_e = \frac{1}{2} L$$

$$L = 2 \times L_e = 1654.6 \text{ mm} = 1.65 \text{ m}$$

(b)

$$P_{cr} = \frac{\bar{\lambda}^2 EI}{L_e^2}$$

$$P_{all} = \frac{P_{cr}}{\text{F.O.S.}}$$

$$P_{cr} = \text{F.O.S.} \times P_{all} \quad \text{if } L_e = \frac{L}{2} = 1.25 \text{ m}$$

So

$$2 \times P_{all} = \frac{\bar{\lambda}^2 EI}{(1.25)^2} = \frac{3.14^2 \times 10^8 \times 1041666.667}{1.5625}$$

$$P_{all} = \frac{3.14^2 \times 10^8 \times 1041666.667}{3.125} = 32.065 \times 10^6 \text{ Pa}$$

Pr 1103

$$A = 3 \times 2 = 1.5 \text{ in}^2$$

$$L = 6 \text{ ft} = 72 \text{ in}$$

$$E = 10.3 \times 10^6 \text{ psi}$$

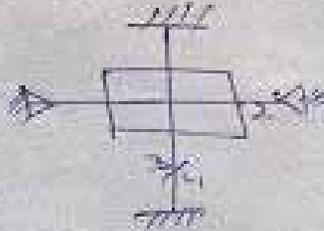
$$F.S. = 2$$

$$Req. P_{safe} = ?$$

Sol  $P_{cr} = \frac{\pi^2 EI}{L^2}$

24

For Hinged ends.

$$I = \frac{2 \times 3^3}{12} = 0.5$$


so  $P_{cr} = \frac{3.14^2 \times 10.3 \times 10^6 \times 0.5 \text{ in}^4}{(72)^2}$

$$= 9794.95 \text{ lb}$$

$$P_{all} = 4897.46$$

FOR Fixed ends

$$L_e = \frac{L}{2} = 36 \text{ in}$$

$$I = \frac{(3)^3 \times 2}{12} = 0.075$$

so  $P_{cr} = \frac{\pi^2 EI}{(L_e)^2} = \frac{(3.14)^2 \times (10.3 \times 10^6) \times 0.075}{(36)^2}$

$$P_{cr} = 5405.16$$

$$P_{all} = \frac{P_{cr}}{F.S.} = \frac{5405.16}{2} = 2702.58 \text{ lb}$$

MASOOD  
AKHTAR  
2004  
B#3

Pb 1101 Given  
 $P_{cr} = 20 \times 10^3 \text{ lb}$      $L = 10' = 120''$   
 $E = 29 \times 10^6 \text{ psi}$   
Req  $b = ?$      $l_e = L$   
 Since  $P_{cr} = \frac{\lambda^2 EI}{l_e^2}$   
 $I = \frac{P_{cr} \times l_e^2}{\lambda^2 E} = \frac{20 \times 10^3 \times (120)^2}{3.14^2 \times 29 \times 10^6} = 1.0072 \text{ in}^4$   
 $I = \frac{b^4}{12} = b^4 \times \frac{1.0072 \times 12}{12 \times 12} \Rightarrow b^4 = 1.0072 \times 12$

---

Pb 1105  $I = \frac{P_{cr} \times l_e^2}{\lambda^2 E} = \frac{20 \times 10^3 \times (120)^2}{3.14^2 \times 29 \times 10^6}$   
 $I = 1.0072$   
 $I \Rightarrow \frac{b^4}{12} \Rightarrow b^4 = I \times 12 = 1.0072 \times 12$   
 $b = 3.04 \text{ in}$

Pr 1106

$C_{40 \times 45}$

Given  $\rightarrow$   $A = 67.3 \text{ mm}^2$

$I = 67.3 \times 10^6 \text{ mm}^4 = 10^6 \times 10^{-12} = 10^{-6} \text{ m}^4$

$r = 109 \text{ mm}$ ,  $E = 200 \times 10^9 \text{ Pa}$

$\sigma_{yp} = 240 \times 10^6 \text{ Pa}$

(a) minimum length =  $L = ?$

(b)  $P_{all}$  when  $L = 12 \text{ m}$  &  $F.S = 2.5$

Sol

we know that

$$\frac{L_e}{R} = \sqrt{\frac{\pi^2 E}{\sigma_{yp}}} = \sqrt{\frac{3.14^2 \times 200 \times 10^9}{240 \times 10^6}} = 90.64$$

$$L_e = 90.64 \times R = 90.64 \times 109 \text{ mm}$$

$$L = 9.880 \text{ m}$$

(b)



$$P_{cr} = \frac{\pi^2 EI}{L_e^2} \quad (L_e = L)$$

$$P_{all} = \frac{P_{cr}}{F.S} \Rightarrow P_{cr} = P_{all} \times F.S$$

$$P_{all} \times F.S = \frac{\pi^2 EI}{L_e^2}$$

$$P_{all} = \frac{3.14^2 \times 200 \times 10^9 \times 67.3 \times 10^6}{2.5 \times (9.88)^2}$$

$$P_{all} = 727 \text{ kN}$$

Prob 1110

Given  $\sigma_{yp} = 200 \text{ MPa}$ ,  $E = 200 \text{ GPa}$

(a) Hinged ends  $\ell = 9 \text{ m}$

(b) built-in end  $\ell = 10 \text{ m}$

(c) built-in  $\ell = 10 \text{ m}$  braced at mid

Reqd  $P_{crit} = ?$

sol  $A = 1550 \text{ mm}^2$   $I = 2657 \times 10^6 \text{ mm}^4$

$r = 152 \text{ mm}$   $r_y = 63 \text{ mm}$

(a)  $\sigma_c = \sqrt{\frac{27 \pi^2 E}{4 \ell^2}} = 101.09$   $\ell = k$

$\frac{\ell}{r} = \frac{9000}{63} = 142.85$

Since  $\ell < \frac{\ell_c}{r}$  So

$S_c = \frac{12 \pi^2 E}{25 \left(\frac{\ell}{r}\right)^2} = \frac{1283.14 \times 200 \times 10^9}{25 (142.85)^2}$

$= 18.05 \text{ MPa}$   $50.4 \text{ MPa}$

Since  $P = S_c \times A$

$= 18.05 \text{ MPa}$

$= 50.4 \times 10^6 \times 1550 \times 10^{-6} \text{ m}^2$

$P = 78200 \text{ N}$

(b) Fixed ends  $\ell = 10 \text{ m}$

So  $\ell_c = 0.7 \ell = 7 \text{ m}$

$\frac{\ell_c}{r} = \frac{7000}{63} = 111.11$

$\ell_c = 0.5 \ell = 5 \text{ m}$

$\frac{\ell_c}{r} = \frac{5000}{63} = 79.2$

Since  $\ell > \frac{\ell_c}{r}$  So

$$S_{wc} = \left( 1 - \frac{(L/r)^2}{2cc^2} \right) \frac{S_{yp}}{F.S}$$

$$F.S = \frac{S}{3} + 3 \frac{(L/r)^2}{cc^2} - \frac{(L/r)^3}{cc^3}$$

$$= \frac{5}{3} + 3 \frac{(99.3)^2}{8 \times 10^4} - \frac{(99.3)^3}{8 \times 10^6}$$

$$= 1.66 + 0.291 - 0.050$$

$$F.S = 1.892 = 1.90$$

So

$$S_{wc} = \left( 1 - \frac{(99.3)^2}{2(10^4)^2} \right) \frac{300 \times 10^6}{1.90}$$

$$= (0.69) \times 200 \text{ MPa}$$

$$= 138 \text{ MPa}$$

$$P_{cr} = S_{wc} A = 138 \times 10^6 / 155 \times 10^{-2}$$

$$= 212 \text{ kN}$$

$$= 212000 \text{ N}$$

②  $L = 5 \text{ m}$ ,  $l_e = 0.7L = 0.7 \times 5 = 3.5 \text{ m}$

$$\frac{L_e}{r} = \frac{3500}{63} = 55.55$$

Since  $l_e > L$ , So

$$S_{wc} = \left( 1 - \frac{(L_e/r)^2}{2cc^2} \right) \frac{S_{yp}}{F.S}$$

$$F.S = \frac{S}{3} + 3 \frac{(L_e/r)^2}{cc^2} - \frac{(L_e/r)^3}{cc^3}$$

$$= 1.66 + 0.20 - 0.02 = 1.84$$

So  $S_{wc} = \left( 1 - \frac{(55.55)^2}{2(10^4)^2} \right) \times \frac{300 \text{ MPa}}{1.84}$

Prob 112  
 Given length =  $L = 5m$   
 Ques Find  $l_e/r$  if  
 (a) Circular section of 80mm radius  
 (b) Square section.

Sol  
 both ends free.

(a) Since  $l_e/r$  is slender member  
 $l_e = \frac{L}{2} = 2.5m$   
 $r = \sqrt{\frac{I}{A}} = \sqrt{\frac{\pi d^4}{64 \times \pi}} = \frac{d}{2} = 40mm$   
 So  $\frac{l_e}{r} = \frac{2500}{40} = 62.5$

(b)  $l_e = \frac{L}{2} = 2.5m = 2500mm$   
 $r = \sqrt{\frac{I}{A}} = \sqrt{\frac{b^4}{12}} = \frac{b}{\sqrt{12}} = 14.4$   
 So  $\frac{l_e}{r} = \frac{2500}{14.4} = 173.6$

Prob 113  
 Given  $L = 12ft = 144in$   
 one end fixed & other hinged

Ques Find  $l_e/r$  if  
 (a) Circular section with 2in rad.  
 (b) 2 in square.

Sol  
 $l_e = 0.7L = 0.7 \times 144 = 100.8in$   
 $r = \frac{d}{2} = \frac{2}{2} = 1$   
 $\frac{l_e}{r} = 100.8in$

(b)  $r = \sqrt{\frac{b^2}{12}} = \sqrt{\frac{2^2}{12}} = 0.72$   
 $\frac{l_e}{r} = \frac{100.8}{0.72} = 140$

Pb 1114 Given  $W_{250 \times 167}$  hinged ends,

$P = 1600 \text{ kN}$

$S_{yp} = 300 \times 10^6 \text{ Pa}$ ,  $E = 200 \text{ GPa}$

$A = 21300 \text{ mm}^2 = 21.3 \times 10^{-3} \text{ m}^2$ ,  $I = 300 \times 10^8 \text{ mm}^4$

Req Length = ?

sol To find length we have to find  $l_e$  first

$$\frac{l_e}{R} = \sqrt{\frac{P}{S_{yp}}}$$

$$l_e = \sqrt{\frac{2 \pi^2 E I}{S_{yp} P}} = 101.07$$

$$S_{cr} = \frac{P}{A} = \frac{1600 \times 10^3}{21300 \times 10^{-6}} = 75117370.9$$

$$S_{cr} = \frac{12 \pi^2 E}{25 \left(\frac{l_e}{R}\right)^2}$$

$$\left(\frac{l_e}{R}\right)^2 = \frac{12 \times 2.14^2 \times 200 \times 10^9}{25 \times 75117370.9}$$

$$(l_e)^2 = 12 \times 4.5796 \times 10^9 \times R^2$$

$$= 633$$

$$l_e = 79.69.0$$

$$L = 7969.0 \text{ mm}$$

$l_e < \frac{L}{R}$

Pb 115 Given  $W_{14 \times 102}$   
 $L_e = 30'$ ,  $S_y = 50 \text{ ksi}$ ,  $E = 29 \times 10^6 \text{ psi}$   
 $A = 24.1 \text{ in}^2$ ,  $I = 692 \text{ in}^4$ ,  $r = 2.48$   
 Req  $P_{safe} = ?$

Sol

$$C_c = \sqrt{\frac{12\pi^2 E}{S_y}} = 100.94 = 103$$

$$\frac{L_e}{r} = \frac{30 \times 12}{2.48} = 145.16$$

Since  $C_c < L_e/r$

$$S_y \cdot S_w = \frac{12\pi^2 E}{23 \left(\frac{L_e}{r}\right)^2}$$

$$S_w = \frac{12 \times 29.14^2 \times 29 \times 10^6 \text{ psi}}{23 \times (145.16)^2}$$

$$S_w = 7079.75 \text{ psi}$$

$$P_{safe} = 170.6 \text{ kips}$$


---

Pb 116 Given  $W_{10 \times 30}$  Fixed ends.  
 Req  $P_{safe}$  (a)  $S_y = 25 \text{ ksi}$   
 $L = 10 \text{ m}$  (b)  $S_y = 200 \text{ MPa}$   
 $A = 6620 \text{ mm}^2$ ,  $r = 39.3 \text{ mm}$

Sol

$$C_c = \sqrt{\frac{12\pi^2 E}{S_y}} = 125.6$$

$$R = 39.3 \text{ mm}$$

$$\frac{L_e}{R} = \frac{10000}{39.3} = 254.45$$

$C_c < L_e/r$

$$S_y \cdot S_w = \frac{12\pi^2 E}{23 \left(\frac{L_e}{r}\right)^2} = 150790 = 105709.6 \text{ N}$$

$$P = S_w \times A = 150790 \times 6620 \times 10^{-6} = 105709.6 \text{ N}$$

Pb 1110 Given  $L = 10m$ ,  $E = 200 \times 10^9 Pa$   
 $E_{yp} = 300 \times 10^6 Pa$ ,  $A = 5670 mm^2$ ,  $r = 16.8 mm$

Req  $P = ?$

$$C_c = \sqrt{\frac{\pi^2 E}{\sigma_{yp}}} = \sqrt{\frac{\pi^2 \times 200 \times 10^9}{300 \times 10^6}}$$

$$C_c = 101.075$$

$$\frac{L}{r} = \frac{10000}{16.8} = 595.23$$

$$C_c < \frac{L}{r} \quad \text{So}$$

$$S_w = \frac{12 \pi^2 E}{23 \left(\frac{L}{r}\right)^2} = \frac{12 \times 3.14^2 \times 200 \times 10^9}{23 (595.23)^2}$$

$$S_w = 2903842.64$$

$$P = S_w \times A = 2903842.64 \times 5670 \times 10^{-6}$$

$$= 16464.2 N$$


---

Pb 1120 Given  $S_{12 \times 5}$

Ⓐ  $L = 1m$     Ⓑ  $L = 2m$   
 $A = 6650 mm^2$ ,  $r = 25 mm$

Req  $P_{safe} = ?$

$$\frac{L}{r} = \frac{L}{r} = \frac{1000}{25} = 40$$

Since  $\frac{L}{r} < 55$

$$S_w = (212 - 1.59 \left(\frac{L}{r}\right)^2) \times 10^6$$

$$S_w = 148.4 MPa$$

$$P = S_w \times A$$

Ⓐ  $P = 966 kN$

(10)  $\frac{L}{R} = \frac{3600}{30} = 120$   
 $\frac{L}{R} > 55$  so  
 $s_w = \frac{372 \times 10^3}{\left(\frac{L}{R}\right)^2} = 25.03 \text{ MPa}$   
 $P = s_w \times A = 25.03 \times 10^6 \times 6650 \times 10^{-6}$   
 $= \underline{\underline{171 \text{ kN}}}$

PB1129 Given (Sixty)  
 (a)  $L = 4 \text{ ft}$  (b)  $L = 10 \text{ ft}$   
 $A = 10.3 \text{ in}^2$ ,  $v = 0.980 \text{ in}$   
Req.  $P_{\text{safe}} = ?$

Sol  
 (a)  $\frac{L}{R} = \frac{4 \times 12}{0.980} = 48.9$   
 $12 < \frac{L}{R} < 44.055$  so  
 For  $v > \text{Cov (arm)}$   
 $s_w = (30.7 - 0.25 \left(\frac{L}{R}\right)) \text{ ksi}$   
 $(30.7 - 0.25 \times 48.9) \text{ ksi}$   
 $s_w = 19.43 \text{ ksi}$   
 $P = 19.43 \times 10^3 \times 10.3 = 200 \text{ kips}$

(b)  $\frac{L}{R} = \frac{10 \times 12}{0.980} = 122.4$   
 so  
 $s_w = \frac{54000}{\left(\frac{L}{R}\right)^2} = \frac{54000}{(122.4)^2} = \frac{54000}{14973.76} \text{ ksi}$   
 $s_w = 3.60 \text{ ksi} \Rightarrow P = s_w \times A$   
 $P = 37.0 \text{ kips}$

Pb 1131  
 150mm x 200mm

(a) 2m      (b) 4m       $E = 11.5 \times 10^9 \text{ Pa}$

So

$$s_w = \frac{0.3 E}{\left(\frac{L}{d}\right)^2} = \frac{0.3 \times 11.5 \times 10^9}{\left(\frac{2}{0.15}\right)^2}$$

$$s_w = 19406250$$

$$P_{safe} = s_w \times A = (19406250 \times (0.2 \times 0.15)) = 582 \text{ kN}$$

(b)

$$s_w = \frac{0.3 E}{\left(\frac{L}{d}\right)^2} = \frac{0.3 \times 11.5 \times 10^9}{\left(\frac{4}{0.15}\right)^2}$$

$$s_w = 9051562.5 \text{ Pa}$$

$$P = 148 \text{ kN}$$


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Pb 1132      (a) 2 x 8 in      (b) 12 ft

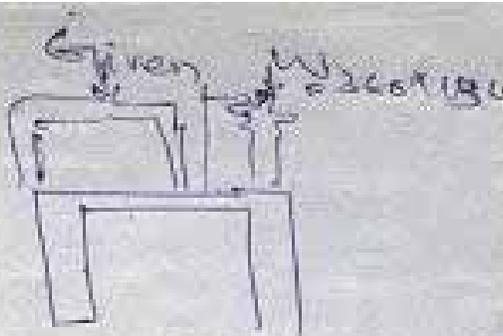
6 ft       $E = 1.6 \times 10^6 \text{ psi}$        $A = 16 \text{ in}^2$

(a)       $d = 8 \text{ in}$

$$s_w = \frac{0.3 \times 1.6 \times 10^6}{\left(\frac{6 \times 12}{8}\right)^2} = 370.3 \text{ psi}$$

$$P = s_w \times A = 5925.9 = \underline{5926 \text{ lb}}$$

Prob 1133



Given  $L_e = 7m$   $W_{300 \times 134}$   
 $P_0 = 400 kN$   $\sigma_{yp} = 250 MPa$   $E = 200 GPa$   
 $A = 7100 mm^2$   $I_x = 2230 \times 10^3 mm^4$   $r = 94 mm$

Sol

$$\frac{L_e}{r} = \frac{7000}{94} = 74.46$$

$$C_c = \sqrt{\frac{2\pi^2 E}{\sigma_{yp}}} = 125.6$$

$C_c > L_e/r$  so

$$F_o = \left(1 - \frac{(L_e/r)^2}{C_c^2}\right) \frac{\sigma_{yp}}{F.S}$$

$$F.S = \frac{F_o}{\sigma_{yp}} + \frac{3(L_e/r)^2}{C_c^2} - \frac{(L_e/r)^4}{C_c^4}$$

$$= \frac{316.6}{250} + \frac{3(74.46)^2}{(125.6)^2} - \frac{(74.46)^4}{(125.6)^4}$$

$$= 1.266 + 0.22 - 0.02$$

$$F.S = 1.466$$

$$C_o = \frac{P_0}{A} = \frac{400 \times 10^3}{7100} = 56.34 MPa$$

$$C_o \leq \sigma_{yp} \left(1 - \frac{(L_e/r)^2}{C_c^2}\right) \frac{250}{1.466} = 110 MPa$$

Now

$$S_{we} = \frac{2P}{A} + \frac{M}{S}$$

$$110.7 \text{ MPa} = \left( \frac{400 \times 10^3 + P}{17100 \times 10^6} \right) + \left( \frac{P \times 0.125 - 400 \times 10^3 \times 0.075}{2320 \times 10^6} \right)$$

$$= \left( \frac{400000 + P}{17100 \times 10^6} \right) + \left( \frac{0.125P - 30000}{2320 \times 10^6} \right)$$

$$110.7 \times 10^6 = 23391812.87 + 0.0000584777 \times 10^6$$

$$50.47P + 53.64P - 12075$$

$$110.7 \times 10^6 = 23370937.37 + 112.11P$$

$$112.11P = 110.7 \times 10^6 - 23370937.37$$

$$P = \underline{\underline{093 \text{ kN}}}$$

Qb 113H

W360 x 122

Given:  $L_e = 10 \text{ m}$ ,  $S_{yp} = 290 \text{ MPa}$ ,  $E = 200 \text{ GPa}$

$A = 15500 \text{ mm}^2$ ,  $r = 302 \text{ mm}$ ,  $S_x = 2010 \times 10^3 \text{ mm}^3$ ,  $r = 63 \text{ mm}$

REQ  $P_c = ?$

Sol

$$\frac{L_e}{r} = \frac{10 \times 10^3}{63} = 158.7$$

$$C_c = \sqrt{\frac{2\pi^2 E}{S_{yp}}} = 116.6$$

$C_c < \frac{L_e}{r}$  So

$$S_{w2} = \frac{12\pi^2 E}{25 \left( \frac{L_e}{r} \right)^2} = \Rightarrow = 40849697.81 \text{ Pa}$$

Now

$$S_w = \frac{FP}{A} + \frac{M}{S_x}$$

$$= \frac{P_c}{15500 \times 10^{-6} \text{ m}^2} + \frac{P_c \times 300 \times 10^{-3} \text{ m}}{2010 \times 10^{-6} \text{ m}^3}$$

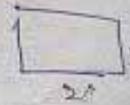
$$= P_c (64.51 + 149.2)$$

$$40049697.7 \frac{\text{N}}{\text{m}^2} = 213.71 P_c$$

$$P_c = 191195.4 \text{ N} = 191 \text{ kN.}$$

~~Pb 135~~ Pb 135 same as above just put  
 $l_e = 4.5 \text{ m}$  & find  $S_w$  then  $P_c$ .

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Pb 136 Given:  $l_e = 5 \text{ ft} = 60 \text{ in}$   

$e = 5 \text{ in}$ ,  $S_y = 36 \text{ in}^3$ ,  $P_c = 11 \times 10^3 \text{ lb}$ ,  $E = 29 \times 10^6 \text{ psi}$

Q. Req:  $P_c = ?$

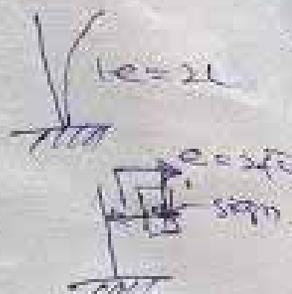
$$\frac{K}{L} = \frac{60}{8.577} = 6.99 \approx 7 \quad \checkmark$$

$$\begin{aligned}
 c_c &= \sqrt{\frac{271E}{s_y p}} = \sqrt{\quad} = 126.03 \\
 \text{Since } c_c &> l_c/r, \quad \text{so} \\
 \text{So } S_w &= \left(1 - \frac{(l_c/r)^2}{2c_c^2}\right) \frac{s_y}{F.S} \\
 F.S &= \frac{3}{8} \frac{s_y}{3} + \frac{3(l_c/r)^2}{8c_c^2} - \frac{(l_c/r)^4}{8c_c^4} \\
 &= 1.66 + 0.309 - 0.07 \\
 F.S &= 1.89 \\
 \text{So } S_w &= \frac{s_y}{F.S} \left(1 - \frac{(l_c/r)^2}{2c_c^2}\right) = \frac{26 \times 10^3 \text{ psi}}{1.89} \\
 S_w &= 13.56 \text{ ksi} \\
 \text{So } S_w &= \frac{E P}{A} + \frac{M}{S_x} \\
 &= \left(\frac{11 \times 10^3 + P_e}{6}\right) + \left(\frac{P_e \times 5}{3}\right) \\
 S_x &= \frac{I}{c} = \frac{D h^3}{12} = \frac{2 \times 27}{12 \times 1.5} = 3 \text{ in}^3 \\
 \text{So } S_w &= \left(\frac{11 \times 10^3 + P_e}{6}\right) + \left(\frac{P_e \times 5}{3}\right) \\
 13.56 \times 10^3 &= 1833.33 + 0.16 P_e = 1.66 P_e \\
 \therefore &= \quad + 1.02 P_e \\
 1.01 P_e &= \quad \\
 P_e &= 5093.7 \text{ N}
 \end{aligned}$$

PD 1137

Given  $L = 8\text{ft}$   
 $L_e = 2L = 16\text{ft}$   
 $e = 2.5\text{ft} = 30\text{in}$   
 $d_o = 4.5\text{in}$ ,  $A = 3.14\text{in}^2$

Req weight of sign  $P = ?$



sol

$$\frac{L_e}{R} = \frac{16 \times 12}{R} \quad \text{Find } R$$

$$R = \sqrt{\frac{I}{A}} = \sqrt{\frac{7.231}{3.14}} = 1.50$$

so  $\frac{L_e}{R} = \frac{16 \times 12}{1.50} = 128$

$$C_c = \sqrt{\frac{2\pi^2 E}{S_{yp}}} = 106.94$$

$C_c < \frac{L_e}{R}$

so  $S_{sw} = \frac{12\pi^2 E}{23(L_e/r)^2} = 9105.22 \text{ psi}$

$$S_{sw} = \frac{E}{A} + \frac{M}{S_x} \quad \text{--- (1)}$$

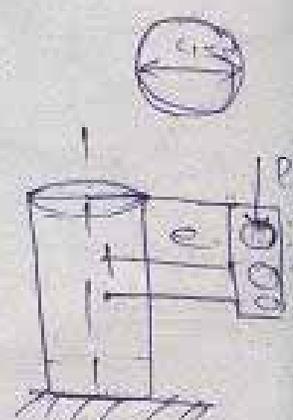
$$S_x = \frac{I}{c} = \frac{\pi r^4}{4} = \frac{\pi r^4}{4r} = \frac{\pi r^3}{4}$$

$$= \frac{\pi r^3}{4} = \frac{3.14 \times (2.25)^3}{4} = 0.94 \text{ in}^3$$

so from eq (1)

$$9105.22 = \left( \frac{P \times 24}{3.14} \right) + \left( \frac{P \times 24}{0.94} \right)$$

$\Rightarrow P = 3035.996$   
 $= 0.310 + 2.600$   
 $\Rightarrow 2P = 9105.22$




Pb 1130

W 260 x 131

Given  $e = L_e = 6m$

$P_1 = 260 kN$       $P_2 = 220 kN$

$S_{yp} = 250 MPa$       $E = 200 GPa$

$A = 17100 mm^2$       $S_x = 2330 \times 10^3 mm^3$

$r = 94 mm$

Q.101

Req =  $e \rightarrow ?$

$\frac{L_e}{r} = \frac{6 \times 1000}{94} = 63.82$

$e_c = \sqrt{\frac{3 \pi E}{S_{yp}}} = \sqrt{\frac{3 \pi \times 200 \times 10^9}{250 \times 10^6}} = 125.6$

$c > \frac{L_e}{r} \quad 30$

$S.W = \left(1 - \frac{(L_e/r)^2}{2e_c^2}\right) \frac{S_{yp}}{F.S}$

$F.S = \frac{5}{3} + \frac{3(L_e/r)}{e_c} - \frac{(L_e/r)^3}{e_c^3}$

$= 1.66 + \frac{3(63.82)}{125.6} - \frac{(63.82)^3}{125.6^3}$

$F.S = 1.66 + 1.519 - 0.016 = 1.03$

So  $S_w = \left(1 - \frac{(63.82)^2}{2(125.6)^2}\right) \frac{250 MPa}{1.03}$

$S_w = 110.97 MPa$

$S_w = \frac{P}{A} + \frac{M}{S_x}$

$= \left(\frac{P_1 + P_2}{A}\right) + \left(\frac{M}{S_x}\right) = \left(\frac{260 \times 10^3 + 220 \times 10^3}{17100 \times 10^{-6}}\right)$

$+ \frac{220 \times 10^3 \times e}{2330 \times 10^3} \Rightarrow$

$110.97 \text{ MPa} = 20070175.94 + 94420600.06 e$   
 $110.97 \times 10^6 - 20070175.94 = 94420600.06 e$   
 $e = 0.96 \text{ m} = \underline{\underline{962 \text{ mm}}}$

Pb 1139      Q 2.45

Given:       $L = 2.2 \text{ m}$ ,  $P = 50 \text{ kN}$ ,  
 $\sigma_{yp} = 300 \text{ MPa}$ ,  $S_x = 140 \text{ cm}^3$   
 $E = 200 \text{ GPa}$ ,  $A = 5690 \text{ mm}^2$   
 $I = 67.3 \times 10^6 \text{ mm}^4$ ,  $r = 19.3 \text{ mm}$   
 $S_x = 442 \times 10^3 \text{ mm}^3$ ,  $S_y = 33.6 \times 10^3 \text{ mm}^3$

Reqd       $e = ?$        $S_x = P/A \Rightarrow 140 \times 10^3 \times 5690 \times 10^{-6} = P$   
 $P = 796600 \text{ N}$

Sol      ( $e = 2.2 \text{ m} \Rightarrow \frac{e}{r} = \frac{2.2 \times 1000}{19.3} = 113.9$ )

$C_c = \sqrt{\frac{2\lambda^2 E}{\sigma_{yp}}} = 101.07$   
 $e < \frac{C_c}{r}$       so

$\sigma_w = \frac{12\lambda^2 E}{22 \left(\frac{e}{r}\right)^2} = 79192764.95 \text{ Pa}$

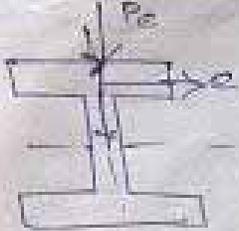
$\sigma_w = \frac{P}{A} + \frac{M}{S_x} = \frac{50 \times 10^3}{5690 \times 10^{-6}} + \frac{50 \times 10^3 \times e}{442 \times 10^3}$   
 $79192764.95 = 8787346.2 + 113.22 \sigma$   
 $\sigma = 622 \text{ MPa}$

$\sigma = 622 \text{ MPa}$

Prob 11.11  
 W14x90 Given  $L = 30 \times 12 = 360 \text{ in}$

$P_o = 65 \times 10^3 \text{ lb} = P_c = 90 \times 10^3 \text{ lb}$  applied  
 $\Rightarrow \sigma_p = 50 \text{ ksi}$   $E = 29 \times 10^6 \text{ psi}$

$A = 26.5 \text{ in}^2$ ,  $I = 999 \text{ in}^4$   
 $S_x = 143 \text{ in}^3$ ,  $\sigma_y = 49.9 \text{ ksi}$   
 $r_y = 3.70 \text{ in}$ ,  $r_x = 6.14 \text{ in}$



$\frac{L_e}{r} = \frac{360}{3.70} = 97.29$

$\phi = \sqrt{\frac{2F_c E}{S_y P}} = 106.94$

$\phi > \frac{L_e}{r}$  so

$\frac{C_m}{F_s} = \frac{5}{16.3} + \frac{3 \left( \frac{L_e}{r} \right)}{\phi^2} - \frac{\left( \frac{L_e}{r} \right)^2}{\phi^2}$

$F_s = 1.66 + 0.34 - 0.054 = 1.95$

$S_w = \left( 1 - \frac{\left( \frac{L_e}{r} \right)^2}{2\phi^2} \right) \frac{S_y P}{F_s}$

$S_w = 15.4 \text{ ksi}$

so  $S_w = \frac{E P}{A} + \frac{M}{S_x}$

$= \frac{65 \times 10^3}{26.5} + \frac{90 \times 10^3 \times 6}{143}$

$\frac{15.4 \times 10^3}{15.425 \times 10^3} = 5049.05 + 629.32$

$\sigma = 15.21 \text{ in}$

$\frac{e}{r} = 20.10$        $\frac{e}{r} = 20.1$

$A \quad F.S = 1.66 + \frac{3(20.10)^2}{8 \times 101.07} - \frac{(20.10)^3}{8 \times (101.07)^3}$

$= 1.66 + 1.499 - 0.000971$

$= 3.15$

So  $S_{10} = \left( 1 - \frac{(20.10)^2}{8(101.07)^2} \right) \times \frac{300 \times 10^5}{3.15}$

$S_{10} = 418267950.2 \text{ Pa}$

$= \frac{P}{A} + \frac{mg}{sg}$

$148787346.2 +$

$\frac{bh^3}{12}$        $\frac{bh^3}{12}$        $\frac{bh^3}{12} + b \times \frac{h^3}{12}$

$\frac{bh^3}{12}$        $\frac{bh^3}{12} + \frac{bh^3}{32}$

$\frac{bh^3}{12}$        $\frac{bh^3 + 3bh^3}{96}$        $\frac{4bh^3}{96}$

$I_x = \frac{bh^3}{24}$

$\frac{I_x}{c} = \frac{bh^3}{24} = \frac{bh^3}{24} \times \frac{h}{h}$

$\frac{h^2}{4} \left[ \frac{bh^2}{c} \right]$

$S = \frac{I}{c}$        $I_x$

$Z = Ay$

$S = \frac{I}{c}$

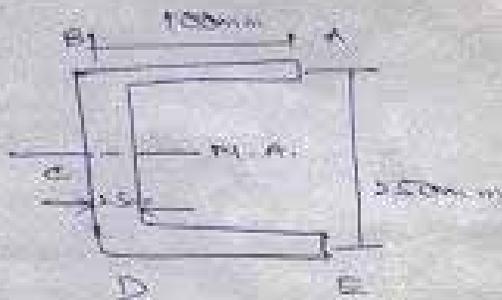
Pd 1321

Given  $V = 2000 \text{ N}$

Find  $\tau$  at  $y = 0$  & draw it

① Shear contour  $\tau = \tau$

Sol



Sol

Sim Q  $\tau = \frac{VQ}{I}$

First of all finding I

$I = E \left( \frac{bh^3}{12} + Ad^2 \right)$

$= 2 \left( \frac{100 \times 125^3}{12} + (100 \times 125)(125)^2 \right) + \left( \frac{250 \times 250^3}{12} + (250 \times 125)(125)^2 \right)$   
 $= 2(130.2 + 3906150) + (3255208.3)$

$1106796.273 \text{ mm}^4$

~~$1106796.273 \times 10^{-12}$~~

$11.06 \times 10^6 \text{ mm}^4$

$I = 11.06 \times 10^{-6} \text{ m}^4$

NOW FOR  $Q_{1-1}$

$Q_{1-1} = A' \bar{y}' = 2.5 \times 125$

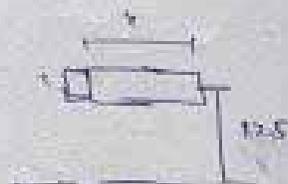
$= 312.5 \text{ mm}^2$

so

$\tau_{1-1} = \frac{VQ_{1-1}}{I} = \frac{2000 \times 312.5 \times 10^{-6} \text{ m}^2}{11.06 \times 10^{-6} \text{ m}^4} = 56.45 \text{ N/m}^2$

$\tau$  at  $x=0$   $\tau_{1-1} = 56.45 \text{ N/m}^2$

$\tau$  at  $x=+1/2$   $\tau_{1-1} = 56.45 \text{ N/m}^2$



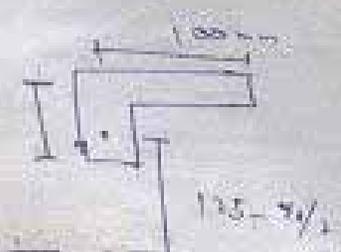
Now  
 $Q_{x-x}$

so  $Q_{x-x} = Q_{A1} + Q_{A2}$

$= A_1 y_1 + A_2 y_2$

$= 100 \times 2.5 \times 125 + 12.5 \times (2.5)^2 \times (125 - y/2)$

$Q_{x-x} = 31250 + 312.5y - 1.25y^2$



$Q_{x-x} = \frac{V Q_{x-x}}{I} = \frac{2000 \times (31250 + 312.5y - 1.25y^2)}{11.06 \times 10^6 \text{ mm}^4}$

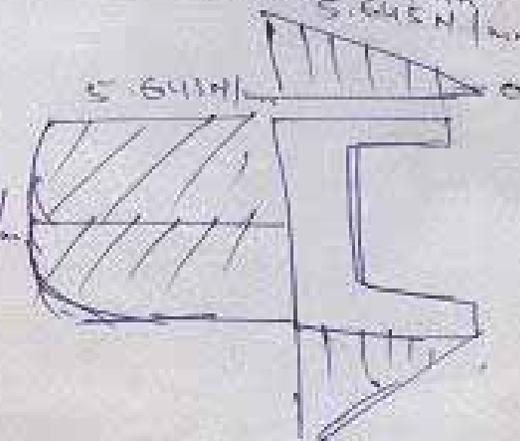
at  $y=0$   $Q_{x-x} = \frac{2000 \times (31250 + 312.5 \times 0 - 1.25 \times 0)}{11.06 \times 10^6 \text{ mm}^4}$

$= 5.645 \text{ N/mm}^2$

at  $y=125$

$Q_{x-x} = \frac{2000 \times (31250 + 312.5 \times 125 - 1.25 \times 125^2)}{11.06 \times 10^6 \text{ mm}^4}$

$= 9.182 \text{ N/mm}^2$   
 $5.645 \text{ N/mm}^2$



so

For  $e = \frac{b^3 b^2}{4I}$

$= \frac{100^3 \times 25^2 \times 25}{4 \times 11.06 \times 10^6}$

$= 35.3 \text{ mm}$

Pb 1322      GIVEN       $t_1 = t_2 = t_3 = 10\text{mm}$   
 $h_1 = 100\text{mm}$ ,       $h_2 = 140\text{mm}$   
 $b_2 = 200\text{mm}$

REQ      Shear Centre =  $P_0$

Sol      Qm C for this section

$$\frac{b_1}{b_2} = \frac{I_2}{I_1}$$

so  $I_1 = \frac{bh^3}{12} = \frac{10 \times 100^3}{12} = 48600000\text{mm}^4$

$$I_2 = \frac{bh^3}{12} = \frac{10 \times 140^3}{12} = 2286666.667\text{mm}^4$$

$b_1 + b_2 = 210\text{mm}$   
 $b_2 = (210\text{mm} - b_1)$

$$\frac{b_1}{(210\text{mm} - b_1)} = \frac{2286666.667}{4860000}$$

$$4860000 b_1 = 480.2 \times 10^6 \text{mm} - 2286666.667 b_1$$

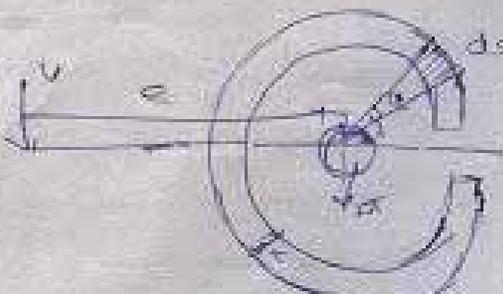
$$7146666.667 b_1 = 480.2 \times 10^6$$

so  $b_2 = 210 - 67.19 = 142.81\text{mm}$        $b_1 = 67.19\text{mm}$

Ph 1323

Given  $\Rightarrow$  radius =  $r$ , thickness =  $t$

REQD  $e = ?$



$ds = r d\phi$

$S = 2\pi r t$

Sol

Since for circular arc

$$q_v = \frac{v t r^2}{I} (1 - \cos \phi)$$

$$q_v = \frac{dF}{ds} \Rightarrow dF = q_v ds$$

$$\int M_o = \int dF \cdot r = \int q_v ds \cdot r = \int \frac{v t r^2}{I} (1 - \cos \phi) r d\phi r$$

$$= \frac{v t r^4}{I} \int_0^{2\pi} (1 - \cos \phi) d\phi = \frac{v t r^4}{I} \left( \phi \Big|_0^{2\pi} - \sin \phi \Big|_0^{2\pi} \right)$$

$$= \frac{v t r^4}{I} (2\pi - (\sin 2\pi - \sin 0)) = \frac{v t r^4}{I} (2\pi)$$

NOW calc.  $I$

$$I_p = I_x + I_y = I_x + I_x = 2I_x$$

$$I_p = 2\pi r(t) r^2 = 2\pi r^3 t$$

$$I_x = \frac{I_p}{2} = \frac{2\pi r^3 t}{2} = \pi r^3 t$$

So

$$\frac{v t r^4}{\pi r^3 t} (2\pi) = 2v r = M_o$$

$\Rightarrow M_o = v e$   
 $v e = 2v r$   
 $e = 2r$

PB 1324

REGDO

$R = \frac{4V}{\pi}$

Sol

$Q = \frac{VQ}{I}$

$Q = \int AY'$

$\cos \phi = \frac{x}{r} \Rightarrow r = \frac{x}{\cos \phi}$

$A = ds \cdot t = r d\phi \cdot t$

so  $Q = \int_0^{\pi} r d\phi \cdot r \cos \phi = \int_0^{\pi} r^2 \cos \phi d\phi$

$Q = r^2 \left| \sin \phi \right|_0^{\pi} = r^2 \sin \pi$

so  $Q = \frac{V \cdot r^2 \sin \pi}{I}$

$dF = qv \cdot ds$

$F = \int qv \cdot ds = \int \frac{V r^2 \sin \phi}{I} \cdot r d\phi$

$= \int_0^{\pi} \frac{V r^3 \sin \phi}{I} d\phi = \frac{V r^3}{I} (-\cos \phi)_0^{\pi}$

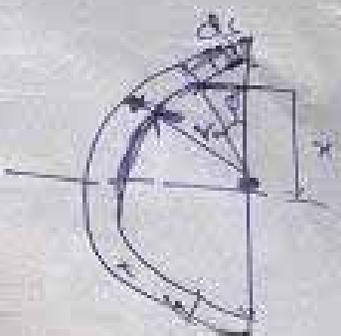
$= \frac{V r^3}{I} (-(-1 - 1)) = \frac{2V r^3}{I}$

For Semi Circle

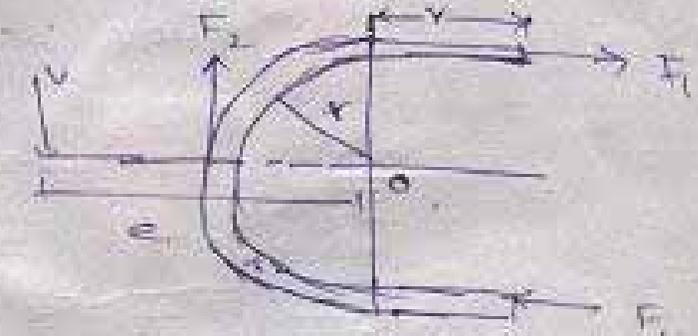
$I = \frac{\pi r^3}{2}$

so  $F = \frac{2V r^3}{\frac{\pi r^3}{2}} = \frac{4V}{\pi}$

$\sum M_0 = 0$   $V_e = \frac{4V}{\pi} \cdot r = \frac{4Vr}{\pi}$



Pb 1325       $R = \frac{V^2}{g}$        $E = \frac{1}{2} \rho V^4 (r+2)$

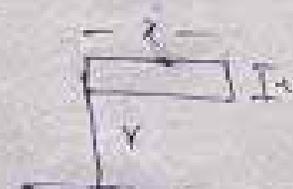


Sol

$Q_{1-2} = \frac{V \cdot A}{I}$

~~$Q_{1-2} = \frac{V \cdot A}{I}$~~

$Q_{1-2} = \frac{V \cdot x \cdot y}{I}$



$F = \int q \cdot dx = \int \frac{V \cdot x \cdot y}{I} dx = \frac{V \cdot y}{I} \int_0^r x dx = \frac{V \cdot y}{I} \cdot \frac{x^2}{2} \Big|_0^r$

$\frac{V \cdot y \cdot r^2}{2I} = \frac{V \cdot y \cdot r^3}{2I} = A$

NO DO FOR  $Q_{1-2}$

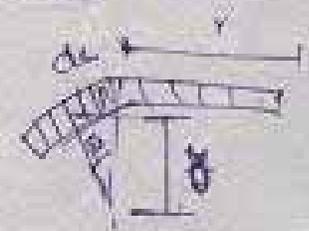
$Q_{1-2} = \dots$

$Q_{1-2} = Q_{1-2} + Q_{2-1}$

$= r \cdot y \cdot x + dx \cdot x \cdot y$

$= r^2 + \int_0^r x \cdot y \cdot dx$

$= r^2 + \frac{1}{2} r^2 \sin \theta \cdot dx = r^2 + \frac{1}{2} r^2 \sin \theta \cdot dx = r^2$



$\cos \theta = \frac{r}{r}$

$\theta = \cos \theta$

$dx = r \cdot d\theta$

$$Q_{2-2} = Vr^2(1 + \sin\theta)$$

$$\text{So } Q_{2-2} = \frac{Vr^2(1 + \sin\theta)}{I}$$

$$F = \int_0^\pi Q_{2-2} r d\theta$$

$$= \int_0^\pi \frac{Vr^2(1 + \sin\theta)}{I} r d\theta$$

$$= \frac{Vr^3}{I} \int_0^\pi (1 + \sin\theta) d\theta$$

$$= \frac{Vr^3}{I} \left( \theta + (-\cos\theta) \right) \Big|_0^\pi$$

$$= \frac{Vr^3}{I} (\pi - (-1 - 1))$$

$$F = \frac{Vr^3}{I} (\pi + 2)$$

Now  $M_o = 0 \rightarrow +ve$

$$V_e = 2F_1 \cdot r + F_2 \cdot r$$

$$V_e = 2 \frac{Vr^3}{2I} r + \frac{Vr^3}{I} (\pi + 2) \cdot r$$

$$\Rightarrow \frac{Vr^4}{I} + \frac{Vr^4}{I} (\pi + 2)$$

$$= \frac{Vr^4}{I} (1 + \pi + 2)$$

$$V_e = \frac{Vr^4}{I} (\pi + 3)$$

$$e = \frac{Vr^4}{I} (\pi + 3)$$

Pb 1326 (9)

Given: 3600 lb

REQ (a) shear flow diagram  
 (b) shear centre.

Sol

Sm @  $V = \frac{VQ}{I}$

$$I = \sum (bh^3 + Ad^2) = 2 \left( \frac{0.10 \times 8^3}{12} + 0.10 \times 2^2 \right) + 2 \left( \frac{4 \times 0.10^3}{12} + 0.10 \times 4 \times 2^2 \right) + \left( \frac{0.10 \times 4^3}{12} + 0.10 \times 4 \times 0 \right)$$

$$= 3.733 + 3.20 + 0.53$$

$$I = 7.46 \text{ in}^4$$

Now  $Q_{1-1}$

$$Q_{1-1} = Ay = 0.10 \times (4 - \frac{x}{2}) = (0.4x - 0.05x^2) \text{ in}^2$$

So  $q_{1-1} = \frac{VQ_{1-1}}{I} = \frac{3600 \times (0.4x - 0.05x^2)}{7.46 \text{ in}^4}$

$$= 482.57 (0.4x - 0.05x^2)$$

$$q_{1-1} = 193.02x - 24.12x^2$$

(a) A  $x=0$   $q_{1-1} = 0$   
 (b) B  $x=8$   $q_{1-1} = 386.04 - 152.64 = 233.40 \text{ lb/in}$

(10)

$$F = \int_0^2 w dx$$

$$= \int_0^2 193.02 x dx - \int_0^2 24.12 x^2 dx$$

$$= \frac{193.02 x^2}{2} \Big|_0^2 - \frac{24.12 x^3}{3} \Big|_0^2$$

$$= 386.04 - 64.52 = 321.52$$

Now taking section 2-2

$$Q_{2-2} = A_1 \bar{y}_1 + A_2 \bar{y}_2$$

$$= 0.10 \times 2 \times 3 + 0.10 \times 2$$

$$Q_{2-2} = 0.6 + 0.2$$

$$w_{2-2} = \frac{V Q_{2-2}}{I}$$

$$w_{2-2} = \frac{3600 (0.6 + 0.2)}{7.96} = 402.57 (0.6 + 0.2)$$

$$w_{2-2} = 209.54 + 96.514 y$$

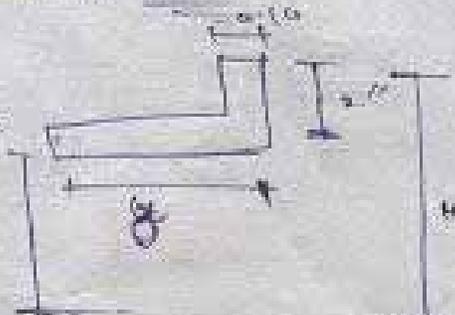
at  $y = 0$   $w_{2-2} = 209.54$

at  $y = 4$   $w_{2-2} = 209.54 + 96.514 \times 4 = 675.50$

$$F_2 = \int_0^4 w dy = \int_0^4 209.54 dy + \int_0^4 96.514 y dy$$

$$= 209.54 y \Big|_0^4 + \frac{96.514 y^2}{2} \Big|_0^4$$

$$= 1150.16 + 772.08$$

$$F_2 = 1930.24$$


$$Q_{z-z} = A_1 y_1 + A_2 y_2 + A_3 y_3$$

$$= 0.10 \times 2 \times 2 + 10 \times 4 \times 2 + 0.10 y (2 - y/2) (2 - y/2)$$

$$= 0.6 + 0.8 + 0.2y - 0.05y^2$$

$$= 1.4 + 0.2y - 0.05y^2$$

$$q_{z-z} = \frac{V Q_{z-z}}{I} = \frac{3600}{746} (0.6 + 0.8 + 0.2y - 0.05y^2)$$

$$= 482.57 (1.4 + 0.2y - 0.05y^2)$$

$$= 675.59 + 96.51y - 24.12y^2$$

$$\frac{d}{dy} q_{z-z} = 0$$

$$q_{z-z} = 675.59 \text{ lb/in}$$

$$\frac{d}{dy} q_{z-z} = 0 \Rightarrow q_{z-z} = 772.13 \text{ lb/in}$$

NOW FOR SHEAR CENTER  
 $M_o = 0 \rightarrow \uparrow +ve$

$$V_e = 2F_1 \times 4 + 2F_2 \times 2$$

$$= 2 \times 321.42 \times 4 + 2 \times 1930.24 \times 2$$

$$e = 2.05 \text{ inches}$$

Pb 1346      GIVEN      inside dia = 20mm  
 inside radius =  $a = 10\text{mm} = 0.01$

$P_i = 60\text{MPa}$   
 $T_{\text{max}} = 90\text{MPa}$

Req       $t = b - a$

Sol      So find  $b$

$$T_{\text{max}} = \frac{b^2 P_i}{(b^2 - a^2)}$$

$$90 \times 10^6 (b^2 - 0.1^2) = b^2 \times 60 \times 10^6$$

$$90b^2 - 2.025 = 60b^2$$

$$30b^2 = 2.025$$

$$b^2 = 0.0675$$

$$b = 0.25$$

So  $t = b - a = 0.25 - 0.1 = 0.15$

① max.  $S_c$  in hoop = ?

Since

②  $S_{c \max} = \frac{-2b^3 P_i}{(b^2 - a^2)}$

$$= \frac{-2 \times (2.5)^3 (3000)}{(2.5^2 - 1.5^2)} = 9775 \text{ psi}$$

③ When  $P_i = 0$  then  $S_c =$

$$S_c = \frac{a^2 P_i - b^2 P_o}{(b^2 - a^2)} + \frac{a^2 b^2 (P_i - P_o)}{(b^2 - a^2) r a^2}$$

$$= \frac{1.5^2 \times 10,000 - 2.5^2 \times 3000}{(2.5^2 - 1.5^2)} + \frac{(1.5^2)(2.5^2)(10,000 - 3000)}{(2.5^2 - 1.5^2) \times (1.5^2)}$$

$$= \frac{27500 - 10750}{4} + 27312.5$$

$$= 937.5 + 27312.5$$

$$= 11075 \text{ psi}$$

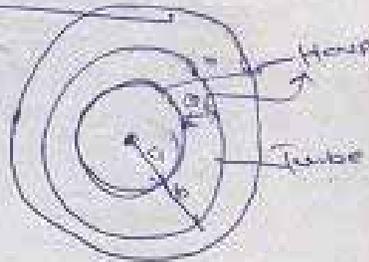
④ For hoop when  $P_o = 0$

so  $S_c = \frac{P_i (a^2 + b^2)}{(b^2 - a^2)} = \frac{3000 (2.5^2 + 4^2)}{(4^2 - 2.5^2)} = 6046.15 \text{ psi}$

Pb 1348 Given

For tube  
 $P_o = 3000 \text{ psi}$   
 $P_i = 10 \times 10^3 \text{ psi}$   
 $a = 1.5 \text{ in}$   
 $b = 2.5 \text{ in}$

For Hoop  
 $P_i = 3000 \text{ psi}$   
 $a = 2.5 \text{ in}$   
 $b = 4 \text{ in}$



Req  $S_c = ?$  when

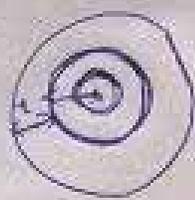
(A)  $S_c$  when  $P_i = 0$   
 (B)  $S_c$  when there is  $P_i$

---

Pb 1349

For tube  
 $t = 1''$ ,  $a = 3''$   
 $b - a = 1''$   
 $b = 4a = 4''$   
 $P_i = 4 \times 10^3 \text{ psi}$   
 Contact

For cylinder  
 $a = 2 \text{ in}$ ,  $b = 3 \text{ in}$   
 $P_o = 4 \times 10^3 \text{ psi}$



$S_c = 14 \times 10^3 \text{ psi}$

Req  $P_i = ?$

Sol

$$S_c = \frac{a^2 P_i - b^2 P_o}{(b^2 - a^2)} + \frac{a^4 b^2 (P_i - P_o)}{(b^2 - a^4) a^2}$$

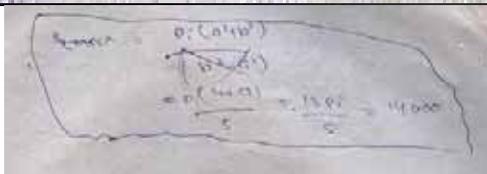
$$14 \times 10^3 = \frac{4 P_i - 9 \times 4 \times 10^3}{(9 - 4)} + \frac{9 (P_i - 4 \times 10^3)}{(9 - 4)}$$

$$P_i = 10923.07 \text{ psi}$$

$$P_i = 59304.6 \text{ psi}$$

$$\frac{1}{5} (13 P_i - 72000) = 14 \times 10^3$$

$$P_i = \frac{5}{13} (14 \times 10^3 + 72000)$$



PD 1350      GIVEN

<p><u>For shaft</u></p> <p><math>a = 0.05m</math>  <math>b = 0.1m</math></p> <p><math>\sigma_{c, max} = 200MPa</math> (contact p)</p> <p>Length of Hub = <math>200mm = 0.2m</math></p> <p><math>\mu_s = 0.40</math></p>	<p><u>For Hub</u></p> <p><math>a = 0.1m</math>  <math>b = 0.15m</math></p>	
---	--	--

Req      Torque = ?

$T = F \times l \times d$

$F = \mu R$

$R = P \times A$

← → Finding P

For the shaft the contact p is external so for shaft

$$\sigma_{c, max} = \frac{-2b^2 p_0}{(b^2 - a^2)} = \frac{-2(0.1^2)(p_0)}{(0.1^2 - 0.05^2)}$$

$$p_0 = \frac{200 \times 10^6 \times (0.1^2 - 0.05^2)}{-2(0.1)^2}$$

= 75 MPa

? For Hub the contact p is internal

so max  $\sigma_c$  is

$\sigma_c = \frac{P_i (b^2 + a^2)}{(b^2 - a^2)} \Rightarrow \frac{200 \times 10^6 (0.15^2 - 0.1^2)}{(0.1^2 + 0.15^2)}$

$\sigma_c = 76.93026 MPa$   
 $\approx 77 MPa$

So the value is TEMPERATURE

$$P = P \cdot A$$

$$= 75 \times 10^6 \times 2 \times 2.14 \times 0.1 \times 0.2$$

$$= 6420000 \text{ N}$$

$$F_s = \mu R = 0.40 \times 6420000$$

$$= 2568000 \text{ N}$$

$$\text{So } T = F_s \times S = 2568000 \times 0.1$$

$$= \underline{\underline{256800 \text{ N.m}}}$$

## SHAPE FACTOR

①  $K = \frac{M_p}{M_y}$

②  $M_p = S_y \sigma$

③  $M_y = S_x \sigma$

④  $K = \frac{S}{I/c}$

$S = \frac{bh^2}{6}$

$S = \frac{bh^2}{4}$

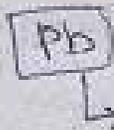
⑤  $Z = Ay'$

⑥  $S = \frac{I_x}{c}$

$I = \frac{bh^3}{12}$  for vertical  $Z_1 \left( \frac{bh^3}{12} \text{ for } + \text{ for } \frac{bh^3}{12} \right)$  for

Horizontal member:

For diamond  $K=2$  for + rectangle 1.5  
for circle = 1.7



Find the shape factor & plastic moment

First of all locate NA

total area =  $A_1 + A_2$

$= 14 \times 1 + 36 \times 1$

$A' = 50 \text{ in}^2$

Now  $Ay' = A_1 y_1 + A_2 y_2$

$Ay' = 14 \times 0.5 + 36 \times 1.9 = 69.1 \text{ in}^2$

$Y' = \frac{69.1}{50} = 13.82 \text{ in}$

Now locate equal area axis

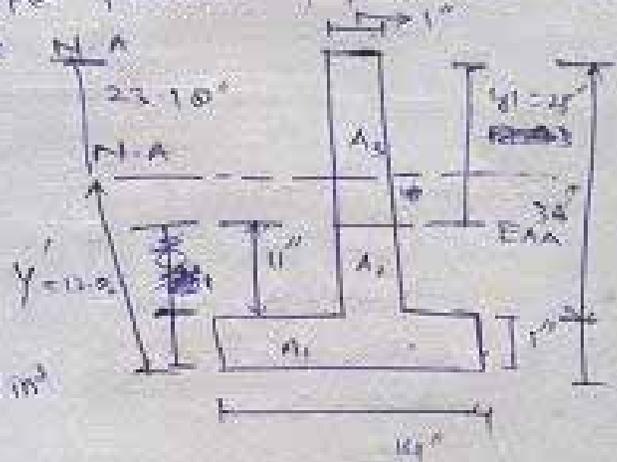
~~$A_1 + A_2 = 14 + 36 = 50$~~   
 ~~$14 \times y_1 + 36 \times y_2 = 50 \times 13.82$~~   
 ~~$14 + y_1 + 36 - y_1 = 50$~~   
 ~~$36 - y_1 = 50 - 14$~~   
 ~~$36 - y_1 = 36$~~   
 ~~$y_1 = 0$~~

$A_2 = A_2 + A_1$

$y_2 = 36 - y_1 + 14$

$2y_2 = 50$

$y_2 = 25$



NOW plastic section modulus

$$Z = \sum A y_{S.A.A}$$

$$= 14 \times 11.5 + 11 \times 10.5 + 25 \times 11.5$$

$$Z = 534 \text{ in}^3$$

NOW moment of inertia

$$I = \frac{14 \times 11.5^3}{3} + \frac{11 \times 10.5^3}{3} + \frac{25 \times 11.5^3}{3} + \left( \frac{14 \times 11.5^3}{12} + 14 \times 11.5 \times (3.25)^2 \right)$$

$$= 4151.633 + 702.332 + 2405.000$$

$$= 7339.045 \text{ in}^4$$

$$S = \frac{I}{c} = \frac{7339.045}{23.125} = 317.3166$$

$$K = \frac{Z}{S} = \frac{534}{317.3166}$$

$$K = 1.683$$

$$M_p = S_y Z = 36 \times 534 = 19224 \text{ lb} \cdot \text{ft}$$

19224 lb-ft

P10

$\sum A = A_1 + A_2 + A_3$   
 $= 40 + 24 + 32 = 96 \text{ in}^2$

$\bar{A} \bar{y} = A_1 \bar{y}_1 + A_2 \bar{y}_2 + A_3 \bar{y}_3$   
 $= 40 \times 1 + 24 \times 6 + 32 \times 10$

$\bar{y} = 9.41 \text{ in}$

NOW Equate areas above and below

$$20 \times 2 + 2(\bar{y}_1) = 2(12 - \bar{y})$$

$$20 \times 2 + A(12 - \bar{y}_1) = 20 \times 2 + 32$$

$$40 + 24 - 2\bar{y}_1 = 20 + 32 \Rightarrow \bar{y}_1 = 0$$

Now calculating plastic section

$$Z = (A y) E_{p/A}$$

$$= 10 \times 5 + 4 \times 2 \times 2 + 8 \times 2 \times 4 + 16 \times 2 \times 9$$

$$= 560 \text{ in}^3$$

In elastic section modulus calculate

$$I = \dots$$

$$I = \left( \frac{bh^3}{12} + Ad^2 \right) + \frac{bh^3}{3} + \frac{bh^3}{3} + \left( \frac{bh^3}{12} + Ad^2 \right)$$

$$= \left( \frac{30 \times 2^3}{12} + 30 \times 2 \times (6.41)^2 \right) + \frac{3 \times 2^3}{3} + \frac{6 \times 2^3}{3} + \left( \frac{16 \times 2^3}{12} + 16 \times 2 \times (7.50)^2 \right)$$

$$I = 3007.23 \text{ in}^4$$

$$S = \frac{I}{c} = \frac{3007.23}{0.50} = 6014.46 \text{ in}^3$$

Shape Factor  $\rightarrow k = \frac{Z}{S} = 1.1201$

$\therefore$  Moment  $= S_y \sigma = 36 \times 560 = 20160 \text{ lb-ft}$

SHAPE FACTOR

①  $k = M_0 / M_y$       ②  $M_0 = S_y \cdot \sigma$       ③  $T_y = S_y \cdot \sigma$   
 ④  $k = \frac{S_y}{S} = \frac{Z}{S}$       ⑤  $Z = A \cdot y$       ⑥  $S = \frac{I}{c}$

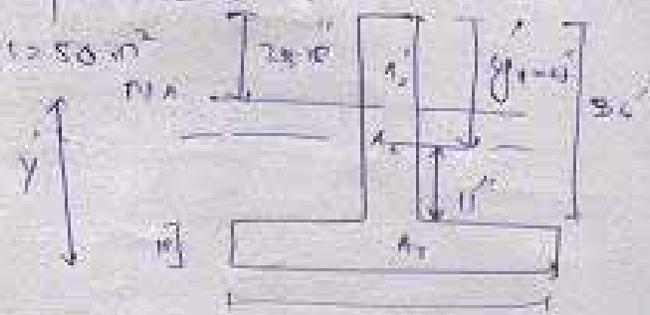
$I = \frac{bh^3}{12}$  for vertical      &       $\left( \frac{bh^3}{12} + Ad^2 \right)$  for horizontal

---

Pb Find S-Factor & Plastic moment

sol First of all find centroidal axis

$A = A_1 + A_2 = 14 \times 14 + 36 \times 12 = 50 \text{ in}^2$   
 $A_y' = A_1 y_1 + A_2 y_2$   
 $= 14 \times 14 \times 7 + 36 \times 12 \times 19$   
 $= 71$   
 $y' = 13.8 \text{ in}$



Now local equal area axis

$A_1 + A_2 = A_2'$   
 $14 \times 14 + (36 - y_1') = y_1'$   
 $14 + 36 = 2y_1'$   
 $y_1' = 25$

Now finding  $S = \frac{A y_1'}{E A A}$

$Z = 14 \times 14 \times 13.8 + 12 \times 12 \times 5.5 + 28 \times 12 \times 5$   
 $= 534 \text{ in}^3$

Now to calculate  $S$  & find  $I_{NA}$

so  $I = \left( \frac{bh^3}{12} + Ad^2 \right) + \frac{bh^3}{12} + \frac{b \cdot h_1^3}{12}$   
 $= \left( \frac{14 \times 14^3}{12} + 14 \times 14 \times 13.8^2 \right) + \left( \frac{1 \times 12 \cdot 0^3}{12} \right) + \left( \frac{1 \times 12 \cdot 12^3}{12} \right)$   
 $= 2405.00 + 702.33 + 362.88 + 4151.6$   
 $I = 6021.81 \text{ in}^4$

$$S = \frac{T}{C} = \frac{7339.04}{2310} = 316 \text{ C.m}^3$$

$$M_p = \frac{3}{5} S = \frac{3 \times 534}{316} = 1.60$$

$$M_p \cdot S_g = 30 \times 534 = 19229 \text{ Nm}$$

QD 2 Find S.Fact  $M_p$

Sol First of all find  $A_y$

$A = 20 \times 2 + 12 \times 2 + 16 \times 2 = 46 \text{ in}^2$

$A_y = A_1 y_1 + A_2 y_2 + A_3 y_3$

$= 20 \times 2 \times 1 + 24 \times 2 + 16 \times 2 \times 5$

$A_y = 712$

$\bar{y} = \frac{712}{46} = 15.478$

Now locate  $E A A$

$20 \times 2 + (12 - y') \times 2 = 2y' + 32$

$40 + 24 - 2y' = 2y' + 32$

$y' = 4$

Finally  $\sum F = E A y \times x$

$Z = 20 \times 2 \times 5 + 4 \times 2 \times 2 + 16 \times 2 \times 9$

$= 568$

$I_{NA} = \left( \frac{b_1 h_1^3}{12} + A_1 d^2 \right) + \frac{b_2 h_2^3}{3} + \frac{b_3 h_3^3}{3} + \left( \frac{b_1 h_1^3}{12} + A_1 d^2 \right)$

$= \left( \frac{20 \times 2^3}{12} + 20 \times 2 \times (15.478 - 1)^2 \right) + \left( \frac{2 \times 591}{3} \right) + \left( \frac{2 \times 6.59^3}{3} \right) + \left( \frac{20 \times 2^3}{12} + 20 \times 2 \times (15.478 - 1)^2 \right)$

$= 1656.891 + 105.84 + 190.794 + 1656.891$

$I = 3007.28$

$S_x = \frac{I}{Z} = \frac{3007.28}{568} = 5.29$

$\sigma = \frac{M}{S} = \frac{568}{5.29} = 107.37 \text{ psi}$

SHEAR FLOW & SHEAR CENTRE

CENTRE

FOR WIDE FLANGE

$$q_v = \frac{VQ}{I}$$

$$q_v = \frac{VQ}{I} \times 2$$

FOR CHANNEL SECT.

$$e = \frac{h^2 b^3}{4I}$$

FOR CIRCULAR

$$q_v = \frac{3Vr^2}{I} (1 - \cos\theta)$$

FOR UNCOUPLED H-SECT

$$\frac{I_1}{M_1} = \frac{I_2}{M_2}$$

$$\frac{I_1}{h} = \frac{I_2}{h}$$

$$\frac{D_1}{D_2} = \frac{I_2}{I_1}$$

MASOOD AKHTAR  
Batch 3  
2007

The image contains several diagrams illustrating shear flow and shear center concepts. At the top, a horizontal line represents the neutral axis with the word 'CENTRE' written above it. Below this, there are three main sections: 1. 'FOR WIDE FLANGE' with a diagram of a wide flange beam showing shear flow arrows pointing outwards from the neutral axis. 2. 'FOR CHANNEL SECT.' with a diagram of a channel section showing shear flow arrows pointing outwards from the web and inwards towards the web in the flanges. 3. 'FOR CIRCULAR' with a diagram of a circular cross-section showing shear flow arrows pointing outwards from the neutral axis. To the right of the channel section diagram is a diagram of an uncoupled H-section with dimensions and labels for its two flanges, showing how they are separated by a gap.

THICK WALLED Cylinder of

$$\frac{r}{r_i} > \frac{1}{10}$$

①  $S_r = A - \frac{B}{r^2}$   
 $S_t = A + \frac{B}{r^2}$

$$A = \frac{a^2 p_i - b^2 p_o}{(b^2 - a^2)}$$

$$B = \frac{a^2 b^2 (p_i - p_o)}{(b^2 - a^2)}$$

So  $S_r = \frac{a^2 p_i - b^2 p_o}{(b^2 - a^2)} - \frac{a^2 b^2 (p_i - p_o)}{(b^2 - a^2) r^2}$

$$S_t = \frac{a^2 p_i - b^2 p_o}{(b^2 - a^2)} + \frac{a^2 b^2 (p_i - p_o)}{(b^2 - a^2) r^2}$$

CASE I INTERNAL P only, i.e.  $p_o = 0$ .

So

$$S_r = \frac{a^2 p_i}{(b^2 - a^2)} \left( 1 - \frac{b^2}{r^2} \right)$$

$$S_r = \frac{a^2 P_i}{(b^2 - a^2)} \left( 1 + \frac{b^2}{r^2} \right)$$

when  $r = a$  then

$$S_r = \frac{a^2 P_i}{(b^2 - a^2)} \left( \frac{a^2 + b^2}{a^2} \right) = -P_i \text{ max}$$

$$\text{if } S_c = \frac{a^2 P_i}{(b^2 - a^2)} \left( \frac{a^2 + b^2}{a^2} \right) = \frac{P_i (a^2 + b^2)}{(b^2 - a^2)} \text{ max}$$

$r = b$

$$S_r = 0 \quad \text{if } S_c = \frac{a^2 P_i}{(b^2 - a^2)} (1 + 1) = \frac{2a^2 P_i}{(b^2 - a^2)} \text{ min}$$

Show  $T_{\text{max}} = -\frac{b^2 P_i}{b^2 - a^2}$

CASE 2  
EXTERNAL P ONLY  $P_i = 0$

$$S_r = \frac{-b^2 P_o}{b^2 - a^2} \left( 1 - \frac{a^2}{r^2} \right)$$

$$S_t = \frac{-b^2 P_o}{b^2 - a^2} \left( 1 + \frac{a^2}{r^2} \right)$$

when  $r = a$

$$S_r = 0 \quad \text{if } S_t = \frac{-b^2 P_o}{(b^2 - a^2)} (2) = -\frac{2b^2 P_o}{(b^2 - a^2)} \text{ max}$$

when  $r = b$

$$S_r = \frac{-b^2 P_o}{(b^2 - a^2)} \left( \frac{b^2 - a^2}{b^2} \right) = -P_o \text{ max}$$

$$S_t = \frac{-b^2 P_o}{(b^2 - a^2)} \left( \frac{b^2 + a^2}{b^2} \right) = -\frac{P_o (b^2 + a^2)}{(b^2 - a^2)}$$